



Understanding Shaft Resistance in Rock Socketed Piles

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Lecture 2 Outline

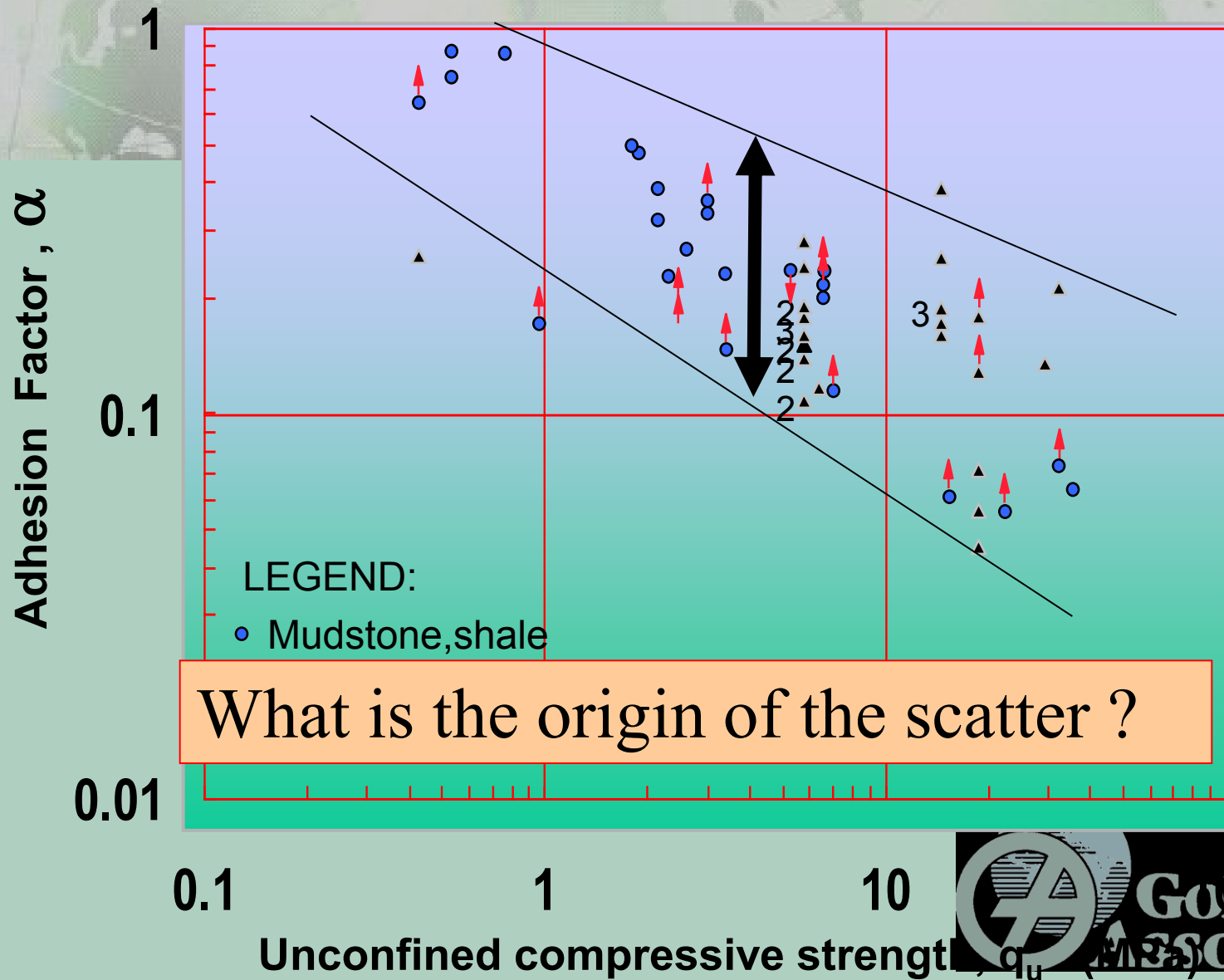
- *Origin of the Scatter (Load test results)*
- *Back to Basics*
- *Some Research Findings*
- *Laboratory Testing of Interfaces*
- *Roughness*
- *From Laboratory to Field*
- *Explaining the Scatter*
- *Summary*

Acknowledgements

- Dr Julian Seidel
- Foundation QA for use of Rocket
- Researchers at Monash University



Shaft Resistance



Parameters affecting Shaft Resistance

Rock

- type, structure, weathering
- strength
- stiffness

Construction

- socket diameter
- socket roughness
- socket cleanliness
- concrete pour
- contractor experience and expertise

Origin of the Scatter

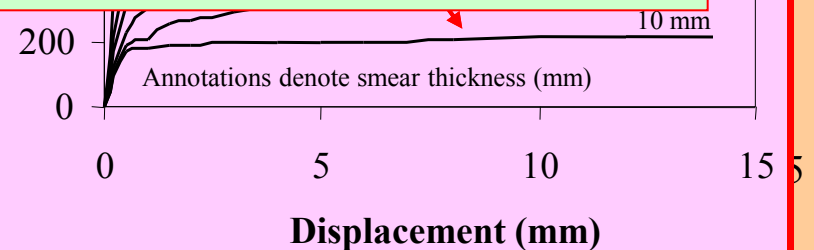
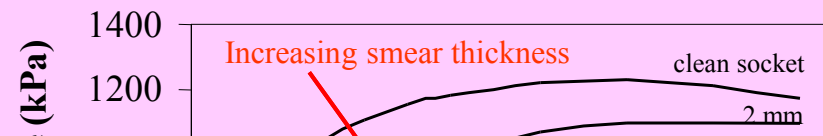
-

- **in** Are their models we can use to predict these effects ?

What engineering properties are required ?

- 
- Cleaning & bonding

Increasing roughness



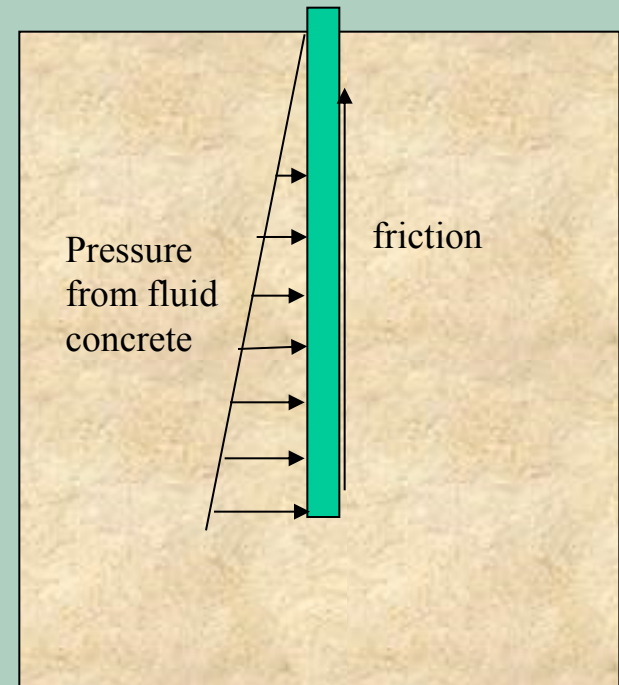
Displacement (mm)

Shaft Resistance - Back to basics

- Shaft resistance is developed through friction (τ) from intimate contact between the concrete of the pile and the rock
- The wet concrete applies a pressure (σ_n) to the socket wall which is locked in when the concrete hardens
- frictional resistance (+ adhesion)

$$\tau = c + \sigma_n \tan \phi$$

must be overcome before slip at the pile/rock interface can occur

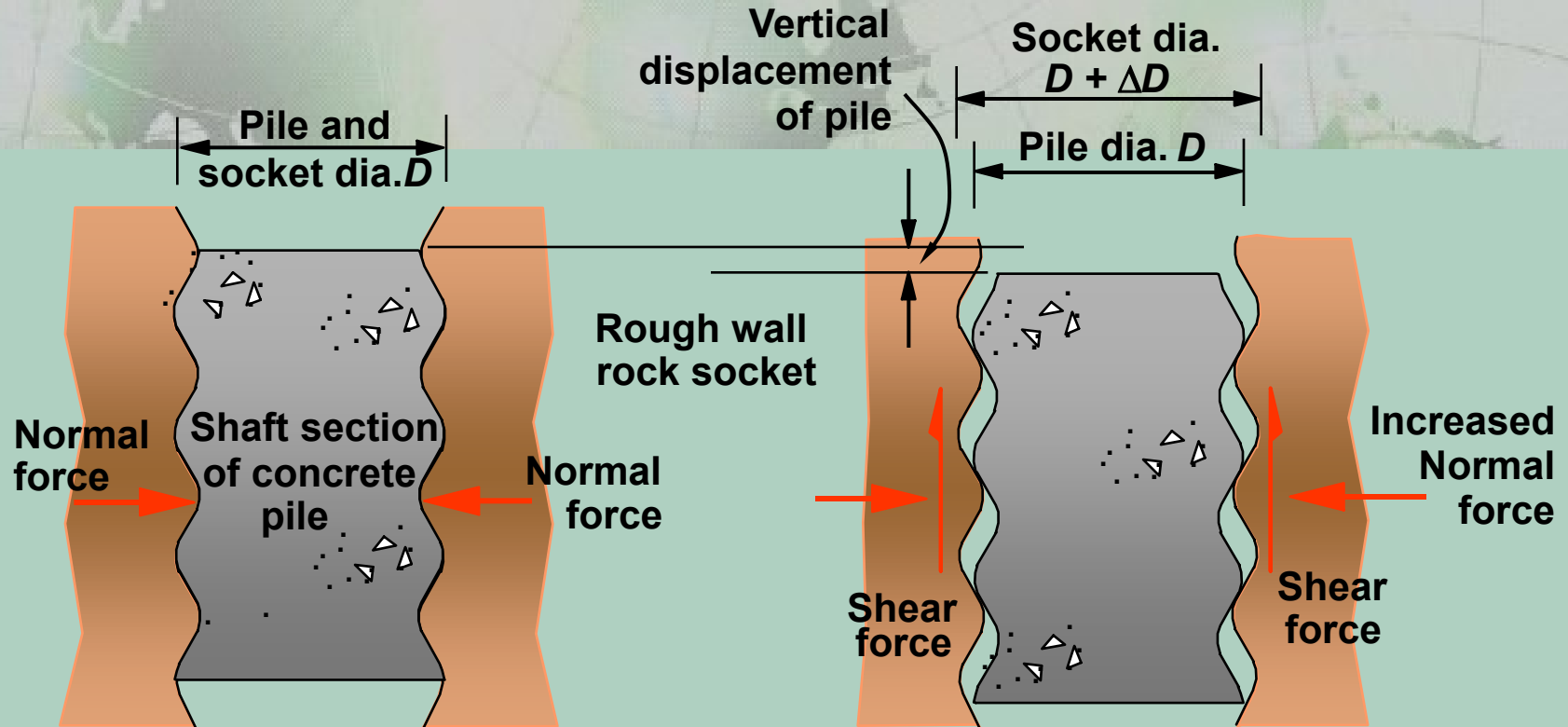


Shaft Resistance - Back to basics

- The rock socket is not smooth, but has undulations referred to as roughness
- For slip to occur at the interface, the socket must dilate
- Thus increasing the normal stress on the interface and the frictional resistance



Socket Dilation



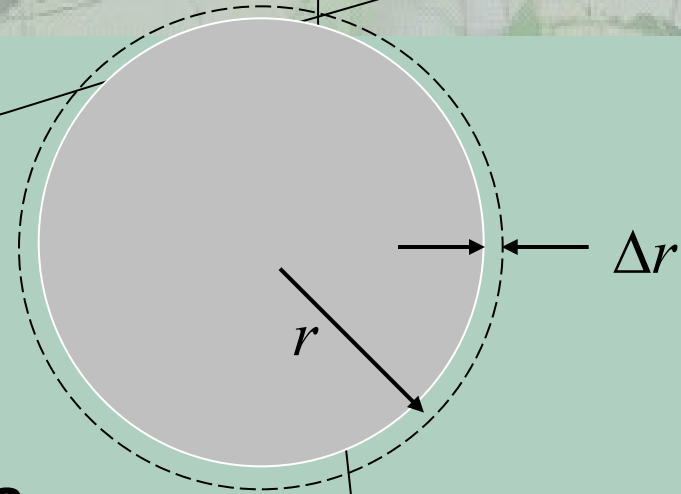
(a) Pile before displacement

(b) Pile after displacement

Constant Normal Stiffness (CNS)

Increase in normal stress

$$\Delta\sigma_n = \frac{E_m \Delta r}{(1 + \nu_m) r}$$



E_m = rock mass Young's modulus

ν_m = rock mass Poisson's ratio

$r = D/2$ = radius of socket

Δr = dilation of socket

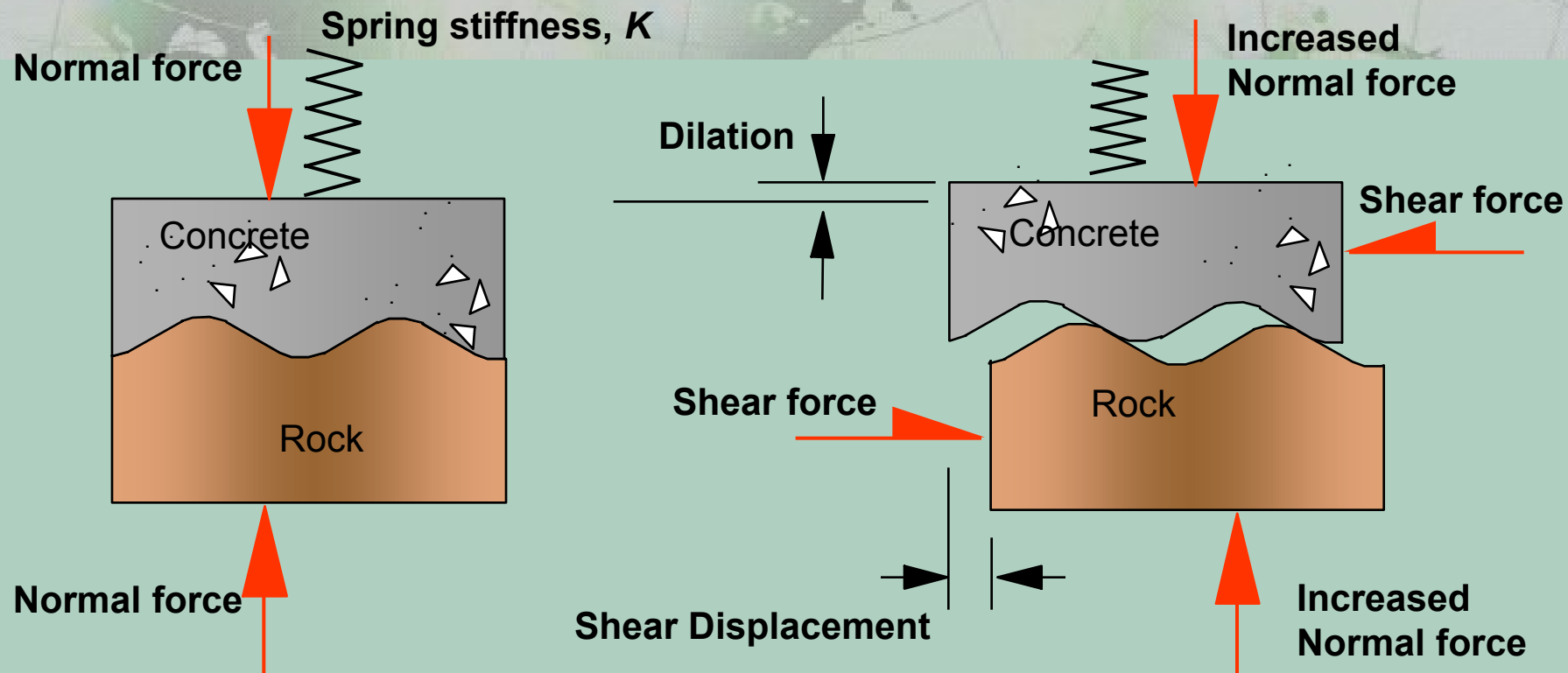
$\Delta\sigma_n$ = change in normal stress

K = normal stiffness

$$K = \frac{\Delta\sigma_n}{\Delta r} = \frac{E_m}{(1 + \nu_m) \cdot r}$$

Modelling socket shear in the laboratory

CNS Direct Shear Tests



(c) Equivalent 2-D model
for before displacement

(d) Equivalent 2-D model
for after displacement

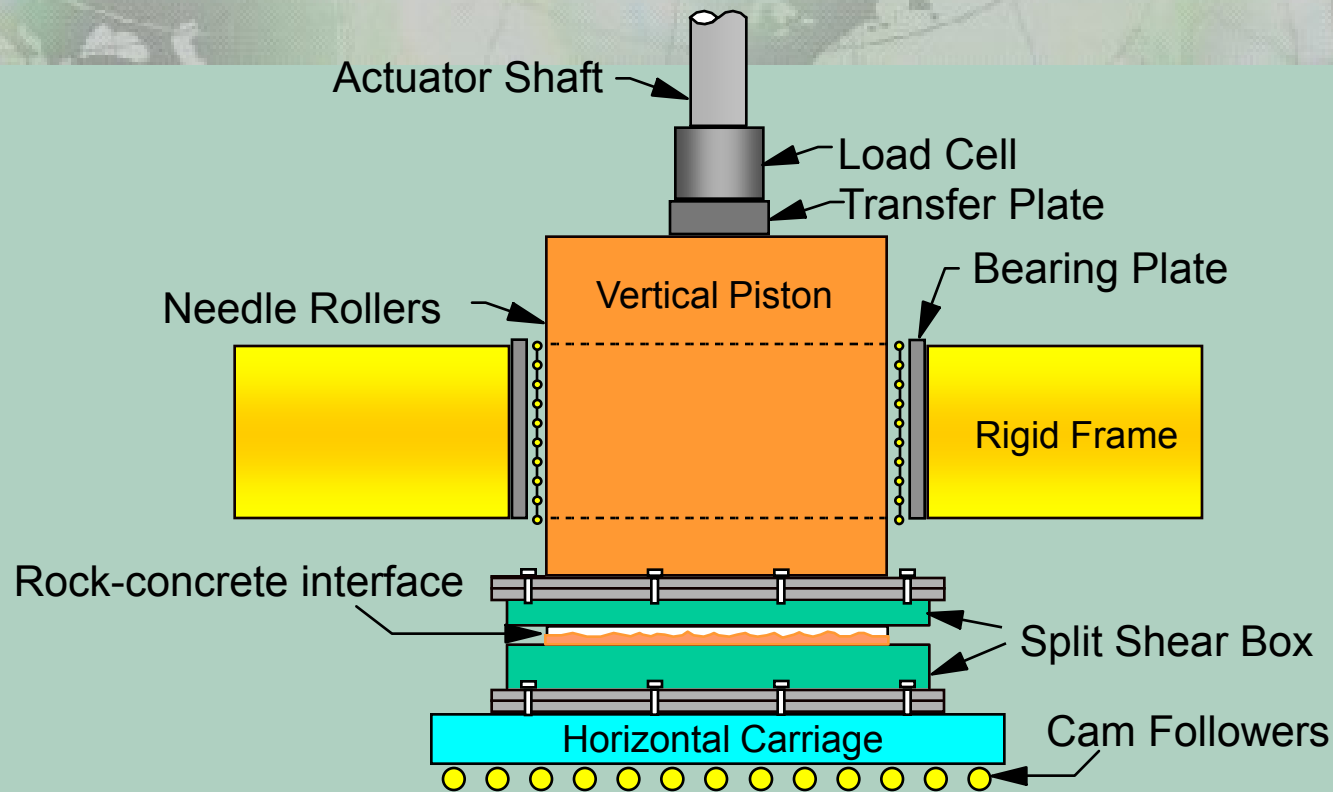


Laboratory Testing : CNS Rig

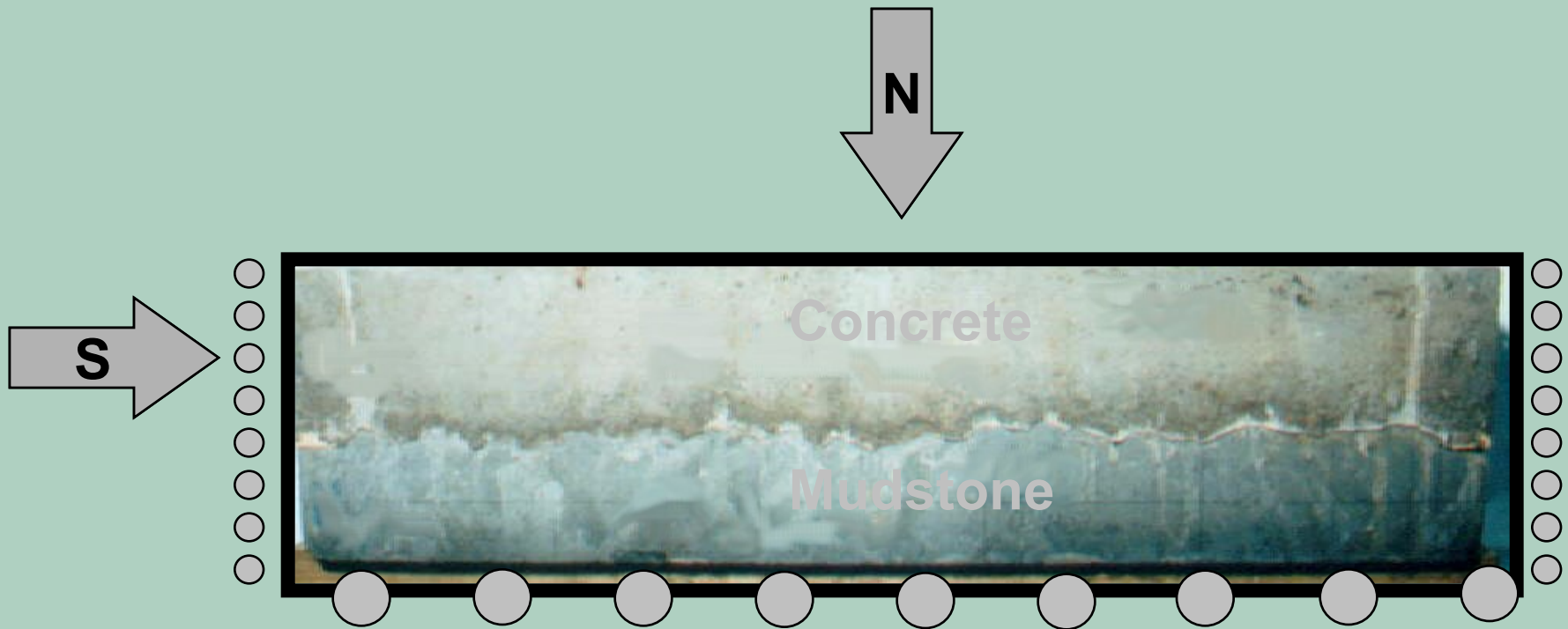


- Computer feedback control
- 250 kN hydraulic actuators for shear and normal stress
- Monotonic and cyclic loading
- Stress or strain control
- Automatic data logging

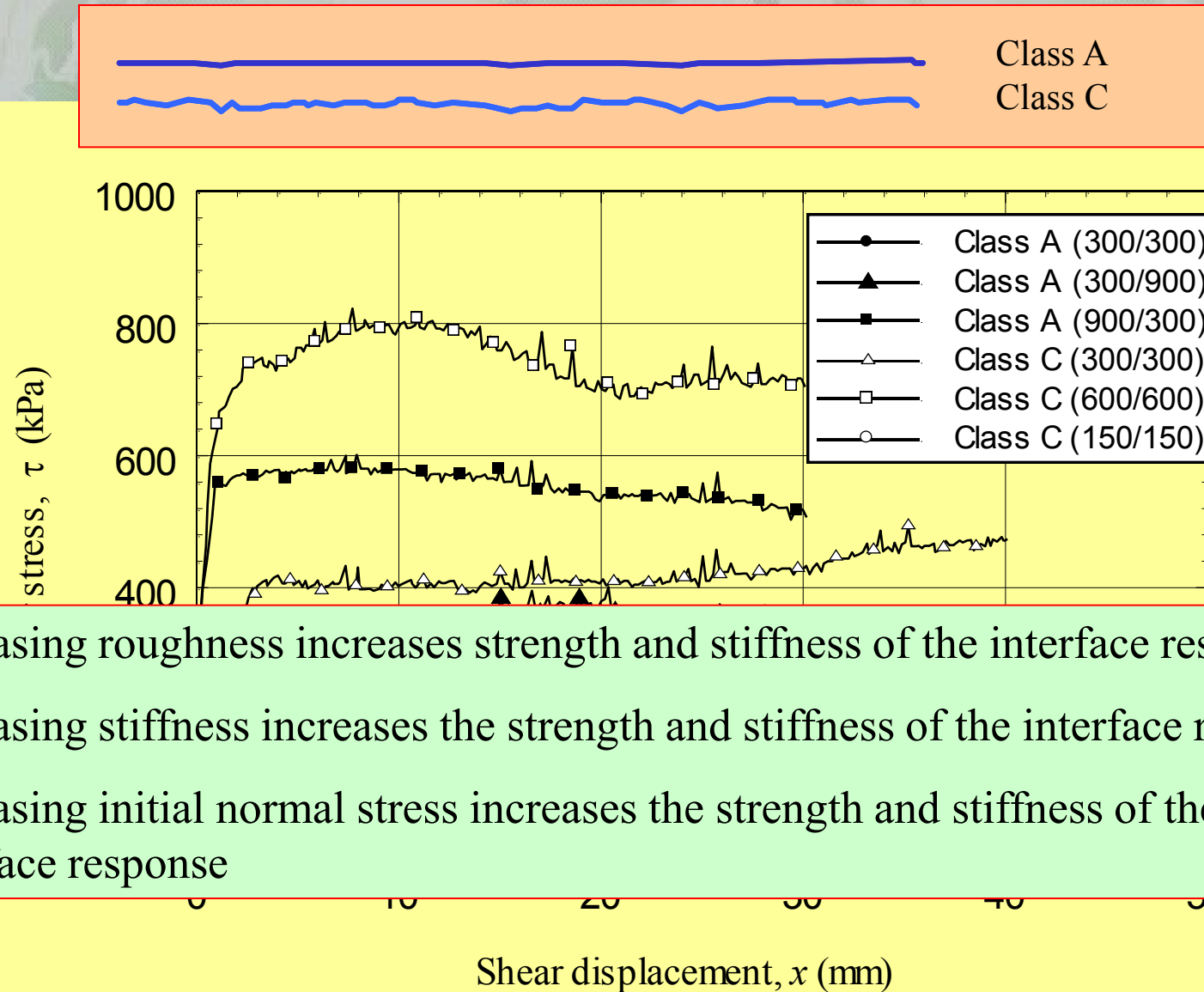
CNS Direct Shear Testing Rig



Rough interface sample prior testing



CNS test results : Impact of roughness, normal stress and stiffness




Increasing roughness increases strength and stiffness of the interface response

Increasing stiffness increases the strength and stiffness of the interface response

Increasing initial normal stress increases the strength and stiffness of the interface response

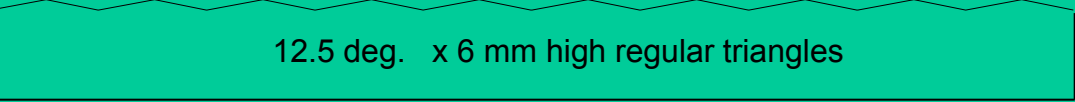
CNS Test Samples - Triangular Asperities




5 deg. x 3.75 mm high regular triangles




10 deg. x 7.5 mm high regular triangles




12.5 deg. x 6 mm high regular triangles




15 deg. x 7.5 mm high regular triangles



17.5 deg. x 9.5 mm high regular triangles

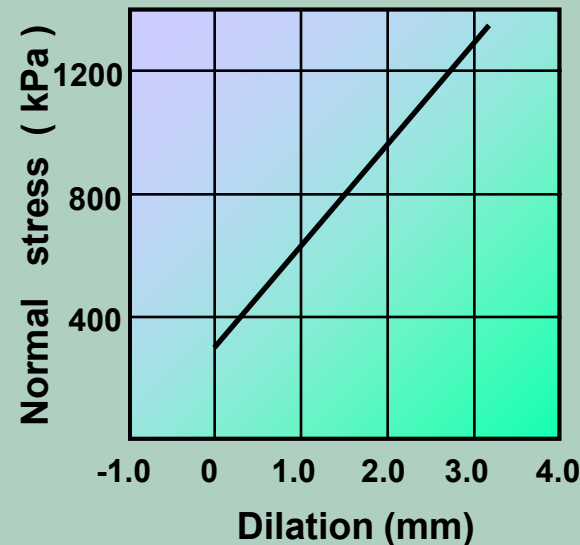
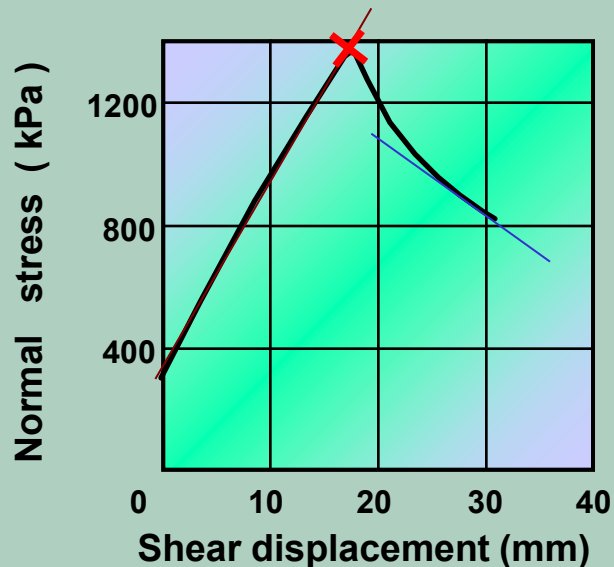
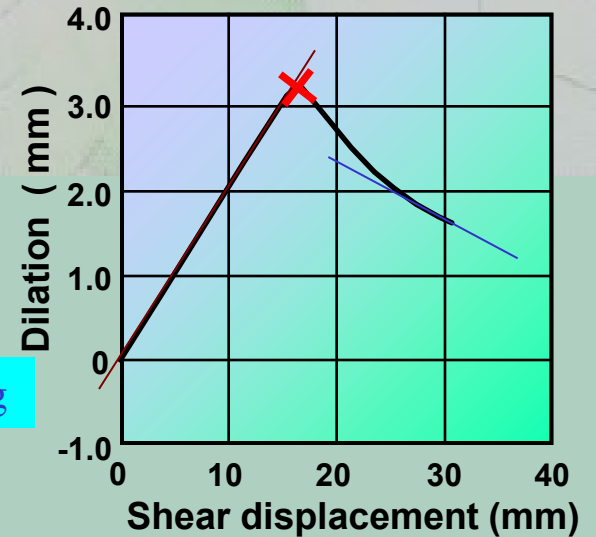
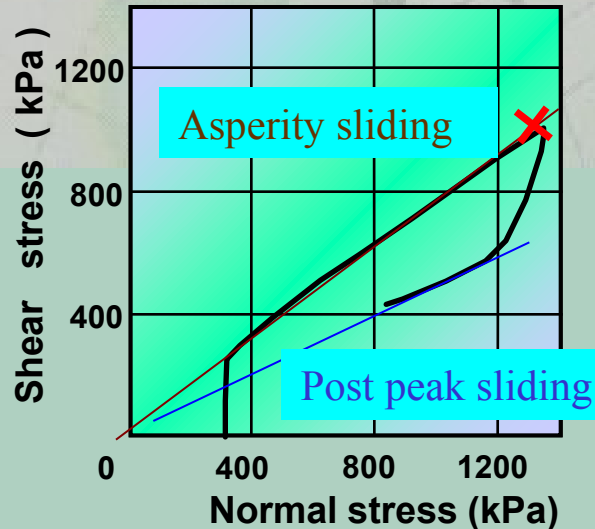
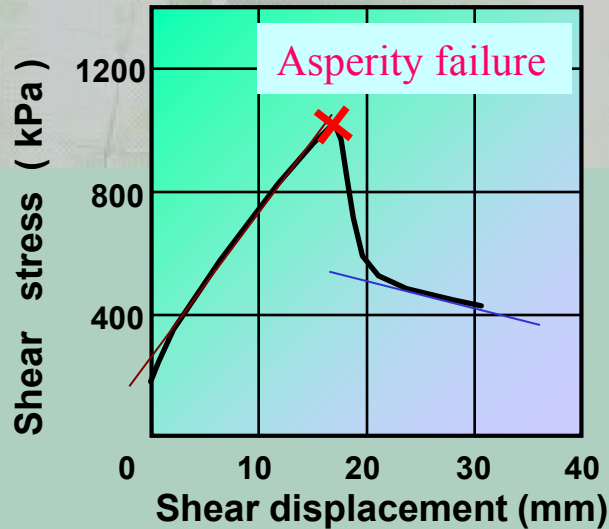


22.5 deg. x 9.5 mm high regular triangles



27.5 deg. x 11.5 mm high regular triangles

Typical Test Results - Triangular Asperities

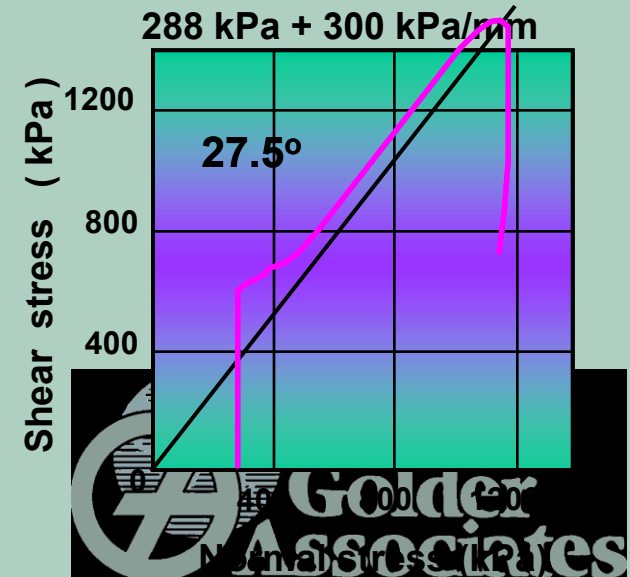
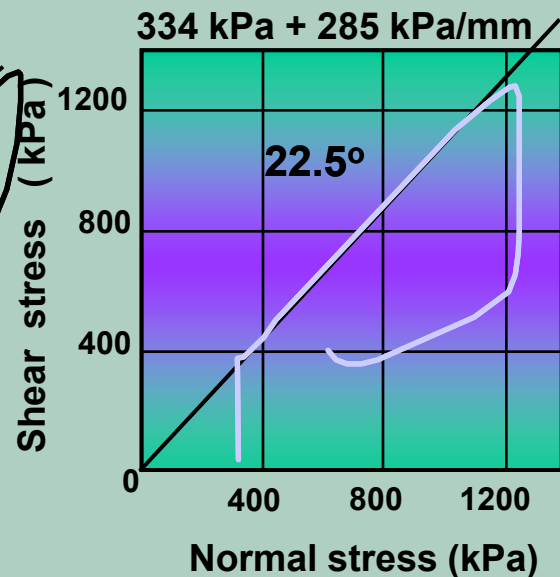
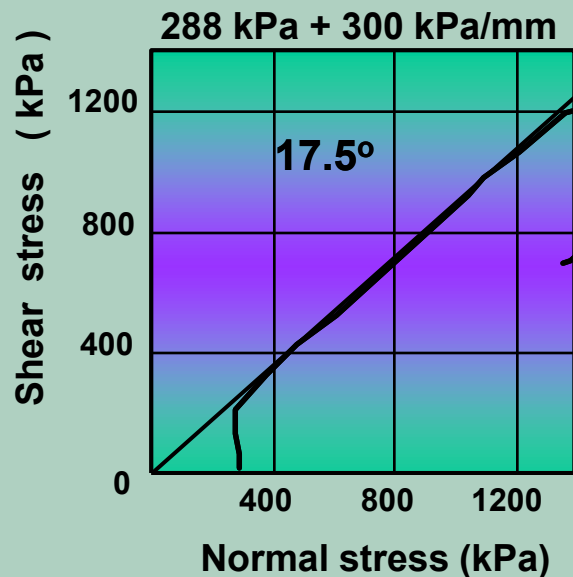
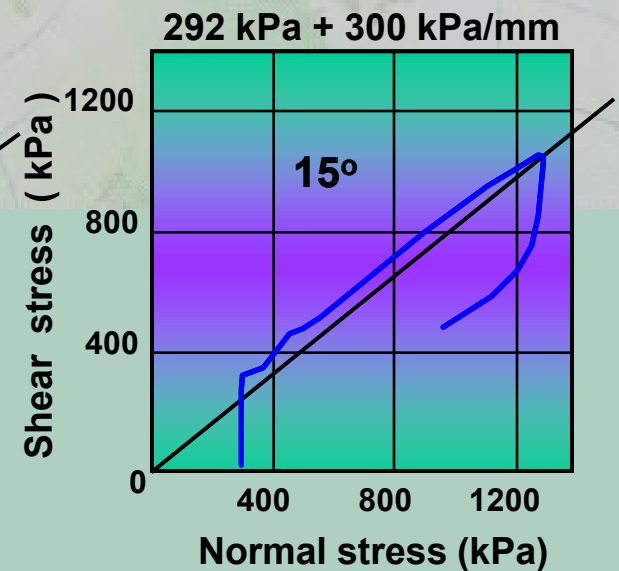
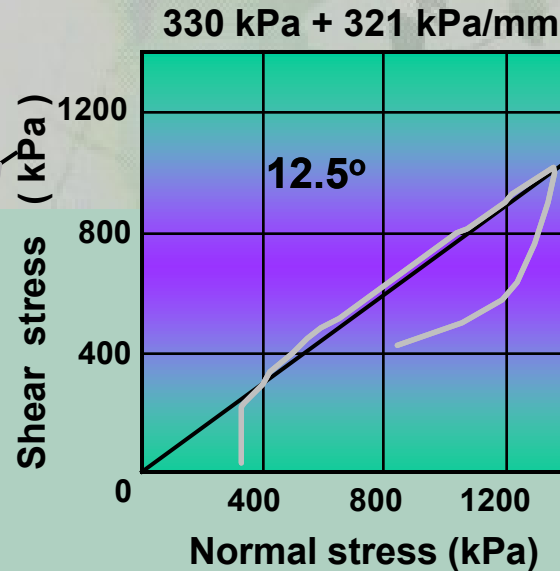
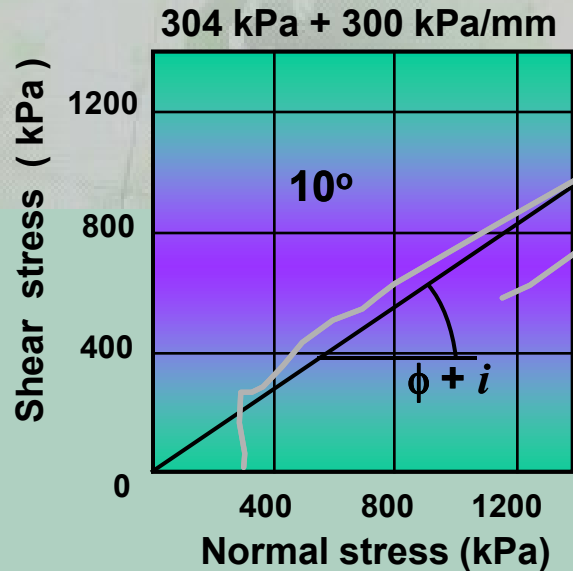


TEST No. M1b 16/4/1992
Johnstone sample 6C
 $12.5^\circ \pm 0^\circ$
Bandsaw cut / gladwrap o'nite

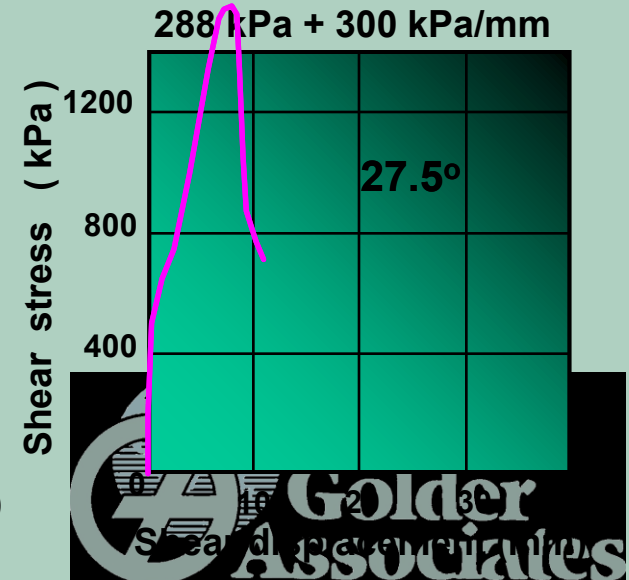
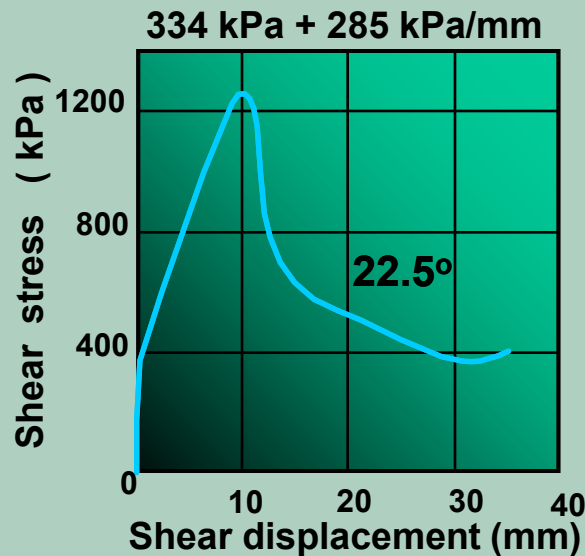
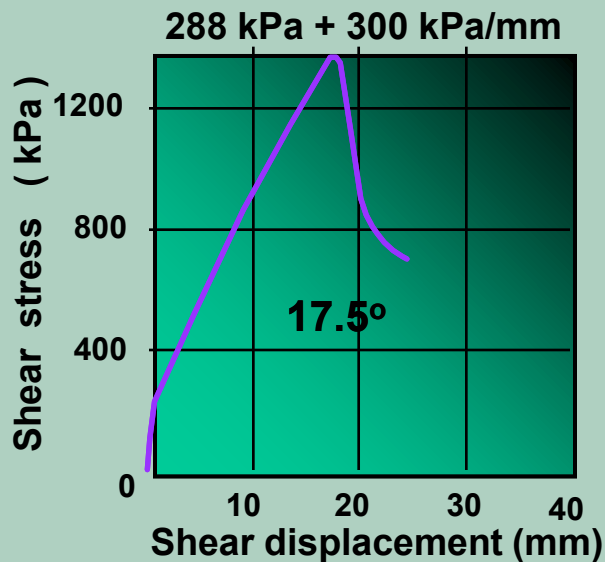
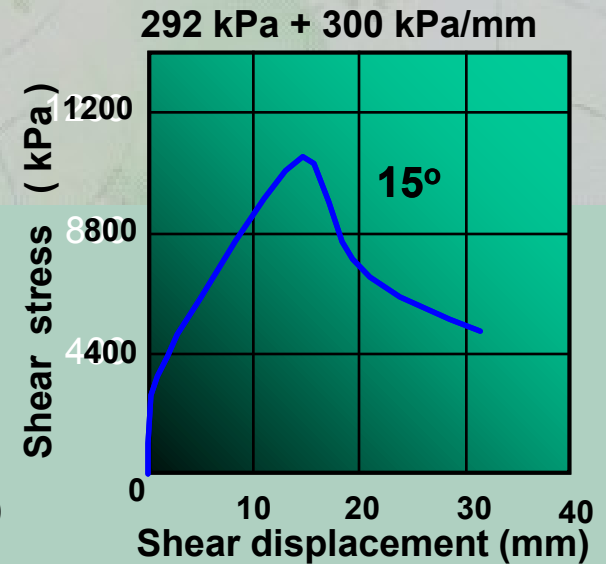
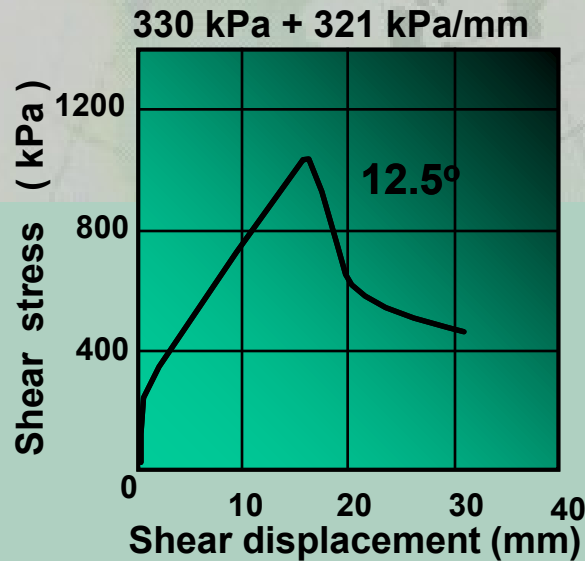
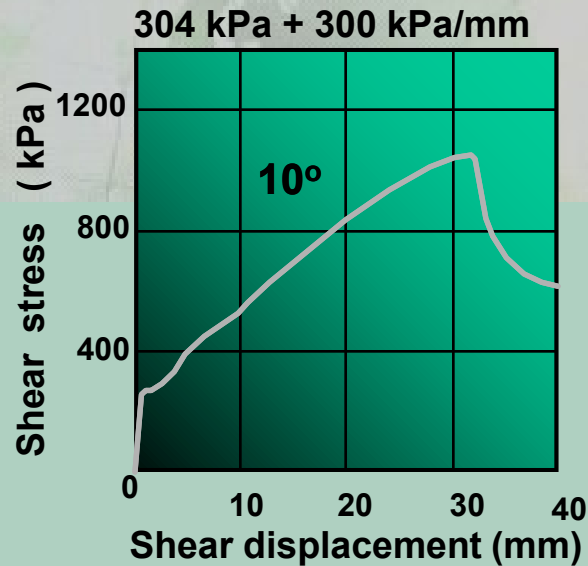


Initial nor. stress = 330 kPa
Normal stiffness = 321 kPa/mm
Shear rate = 0.5 mm/min

Summary 'A' for Triangular Asperities



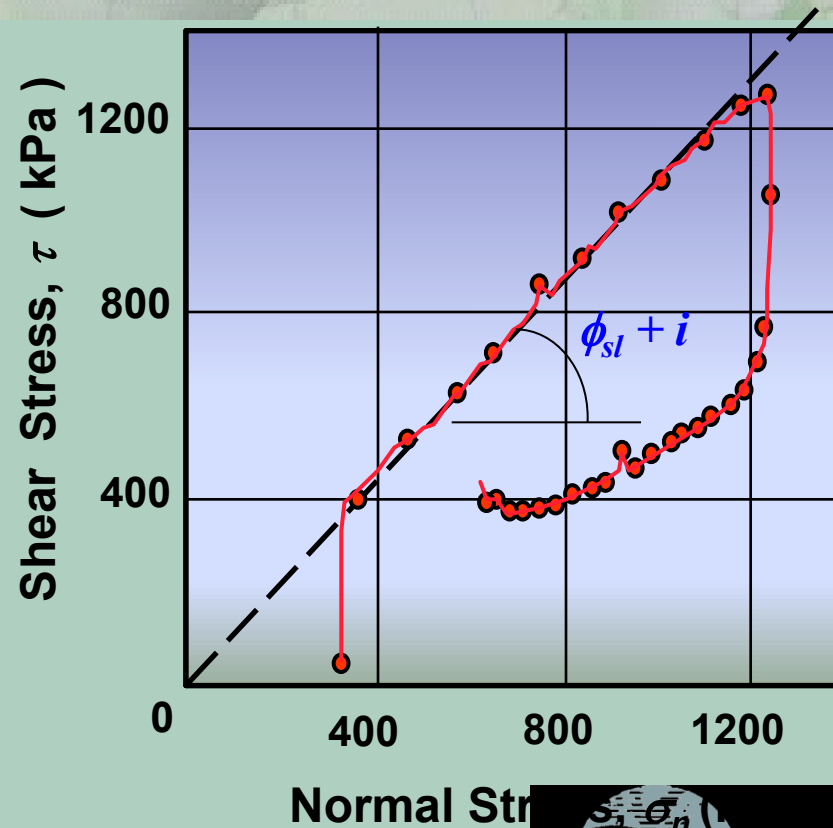
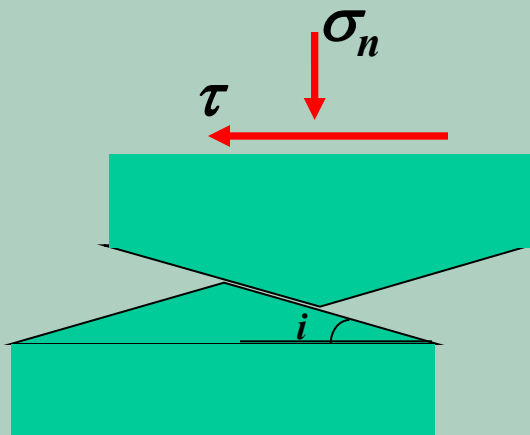
Summary 'B' for Triangular Asperities



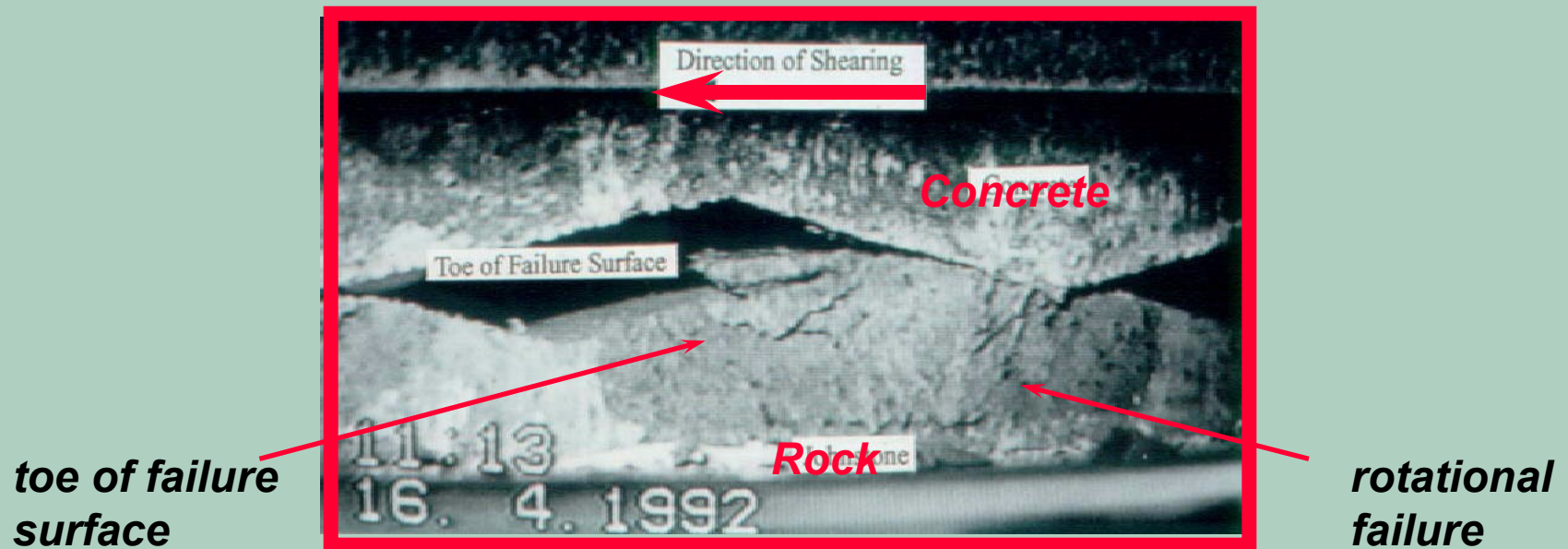
Simple Sliding Model



$$\tau = \sigma_n \tan(\phi_{sl} + i)$$



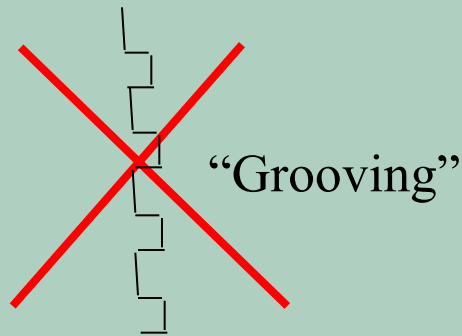
Video image of regular 12.5° siltstone asperity at failure



At a critical shear displacement, the asperity can no longer support the applied load and fails.

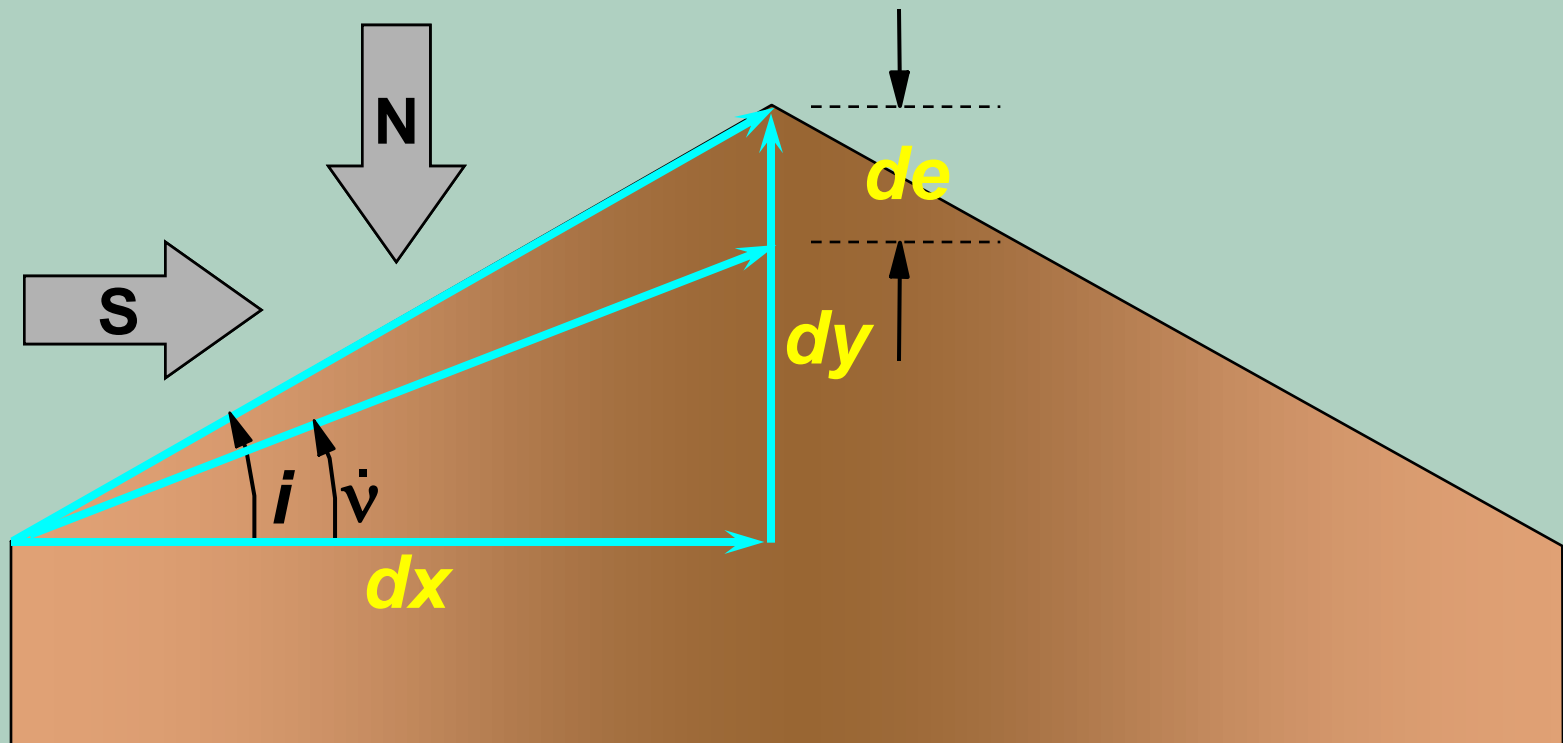
Effective Roughness

If asperities are too steep, there will be no sliding and no dilation. As a result, the interface may have lower shear strength and will behave in a more brittle manner. There is an optimal level of roughness, beyond which no improvement to shear performance will occur.



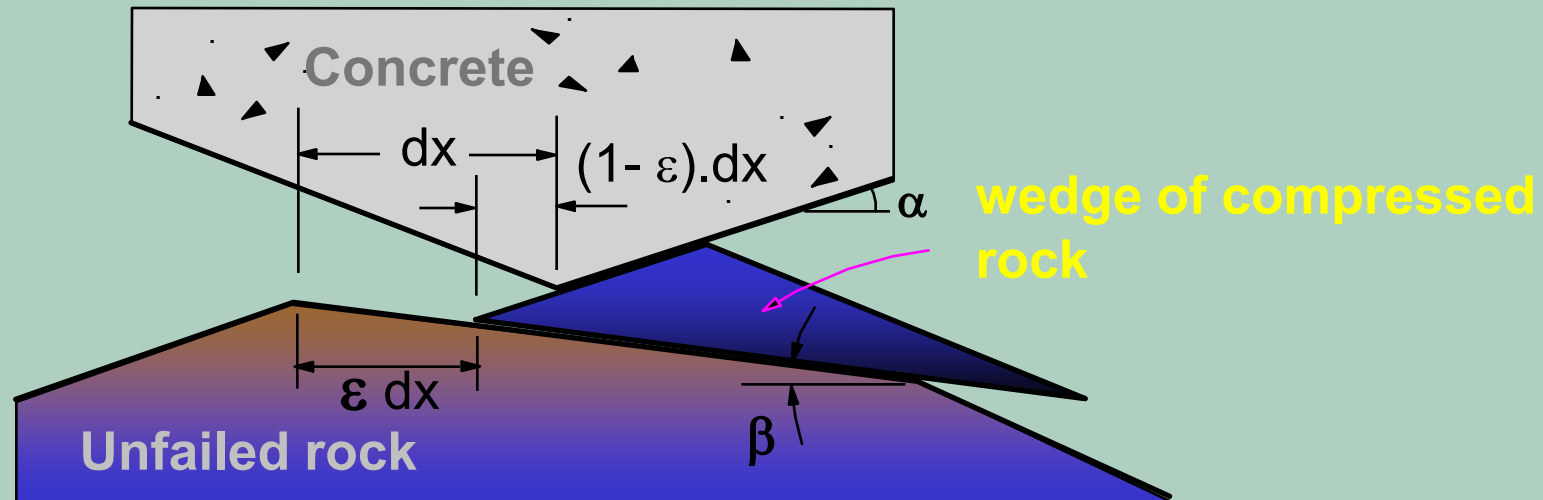
Deformation and dilation

Asperities deform under load and reduces dilation
(to less than the asperity angle)



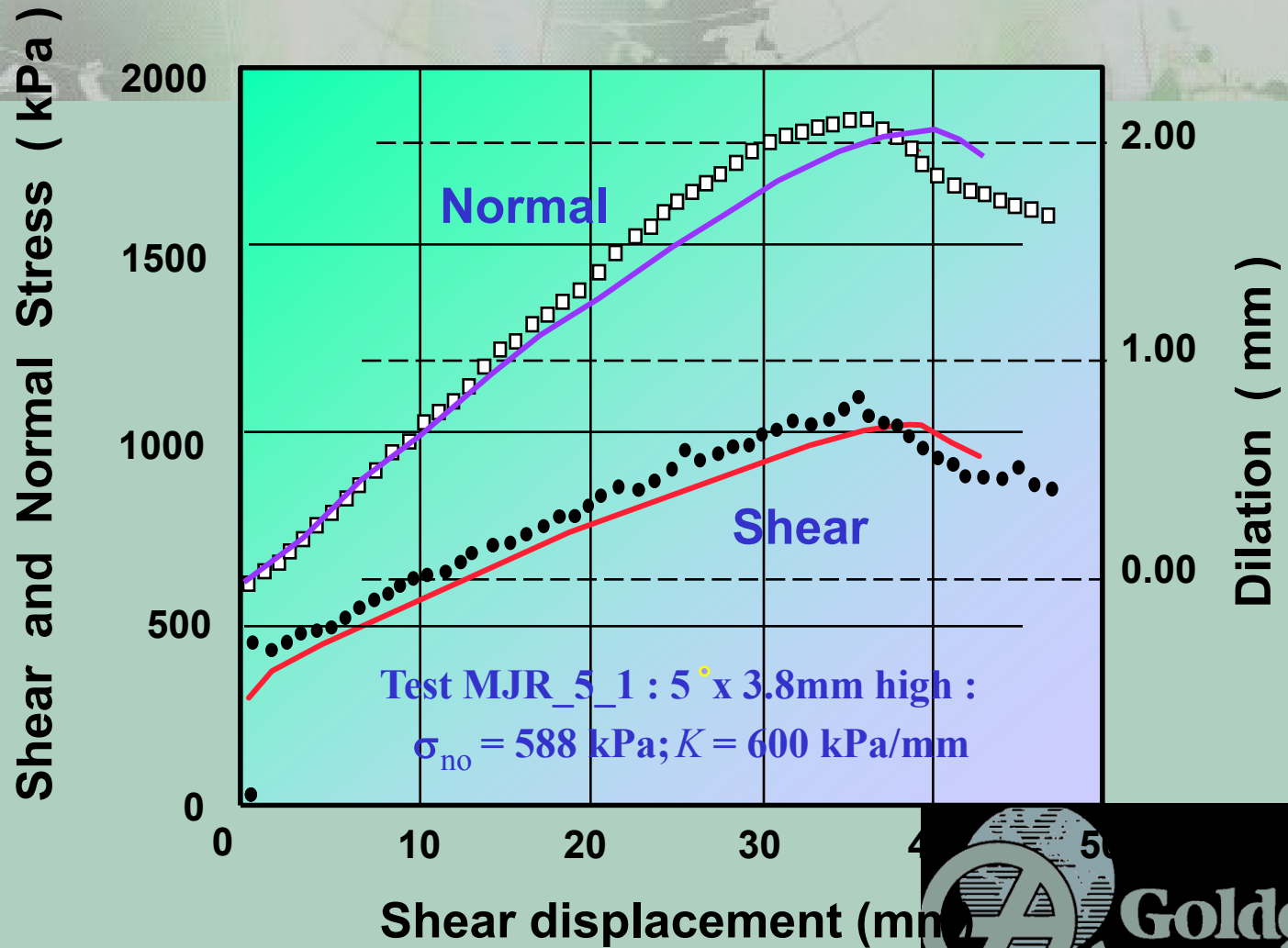
Behaviour after Failure

- After failure of the asperity, there is a wedge of compressed rubble which effectively acts as a door-stopper

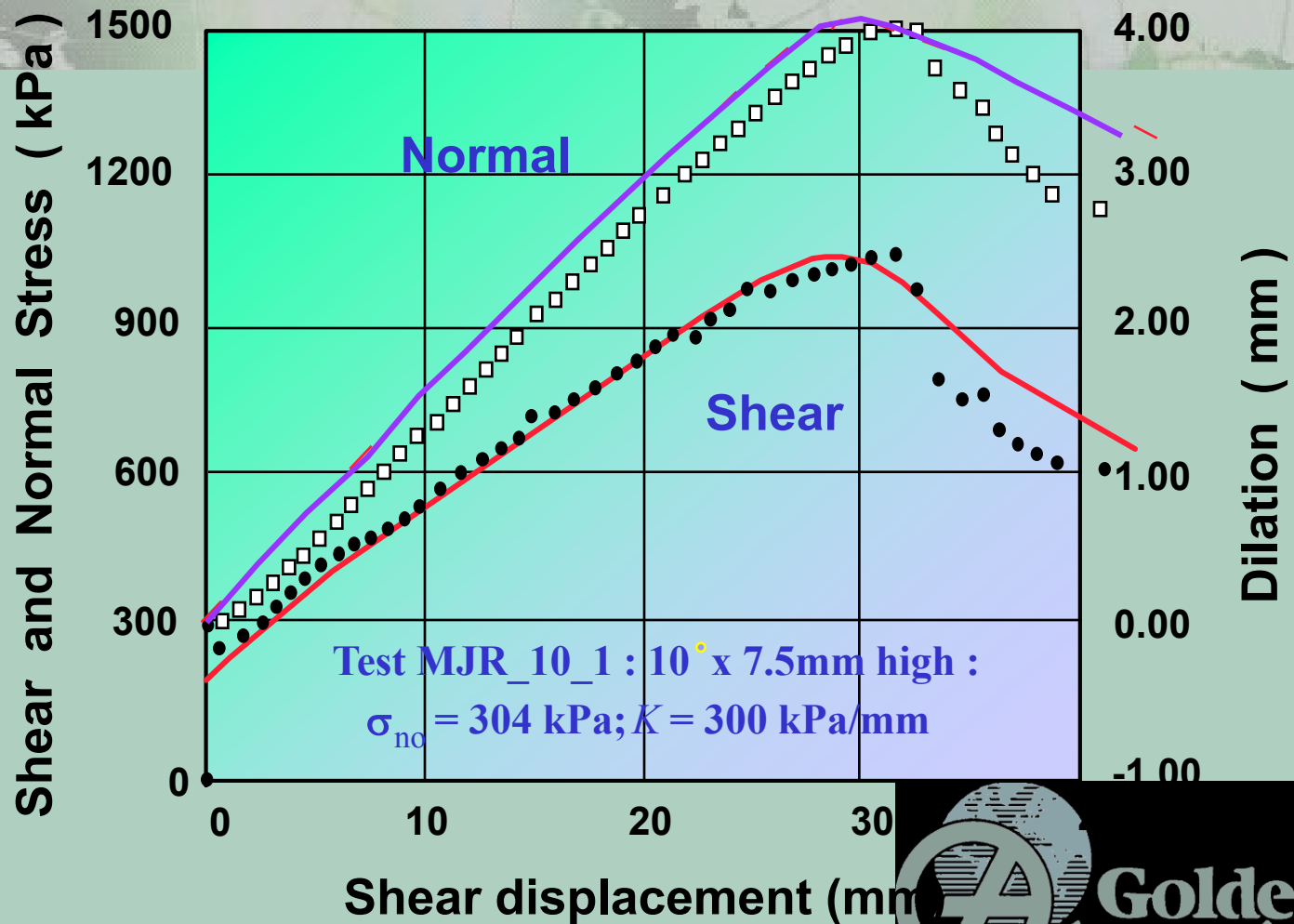


Relative movement occurs between both the concrete and the wedge and the wedge and unfailed material. This results in a residual strength greater than the residual strength of the rock.

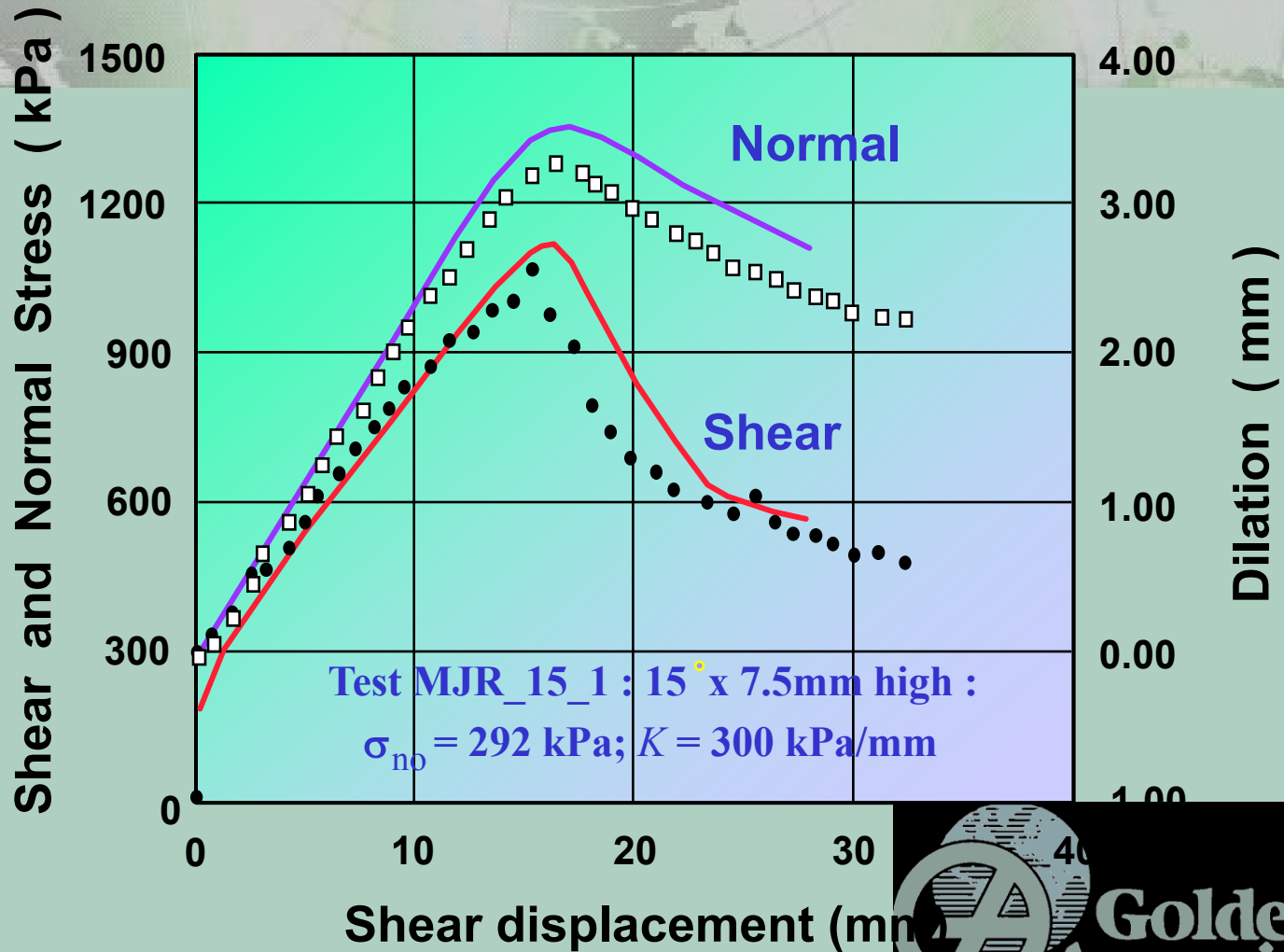
5° asperity profile - measured vs predicted



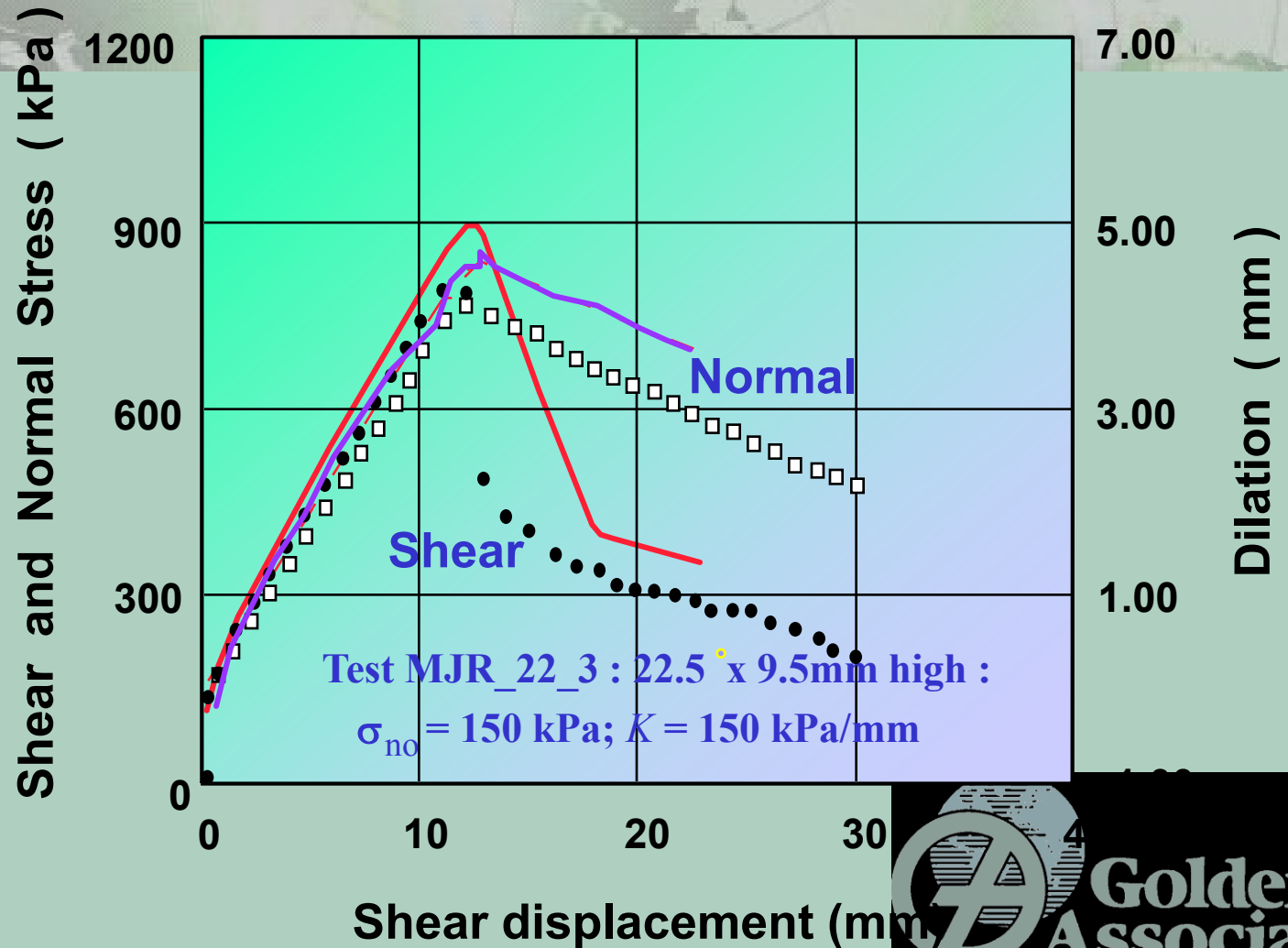
10° asperity profile - measured vs predicted



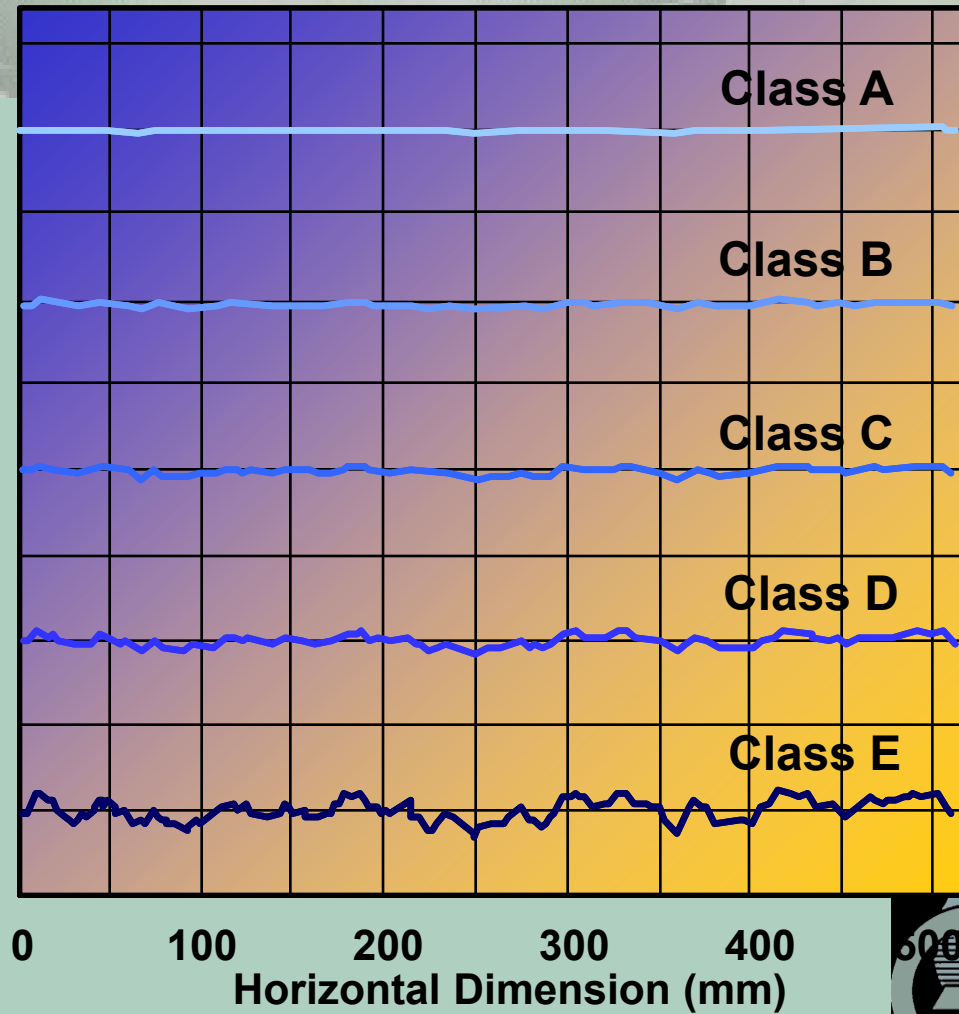
15° asperity - measured vs predicted



22.5° asperity - measured vs predicted

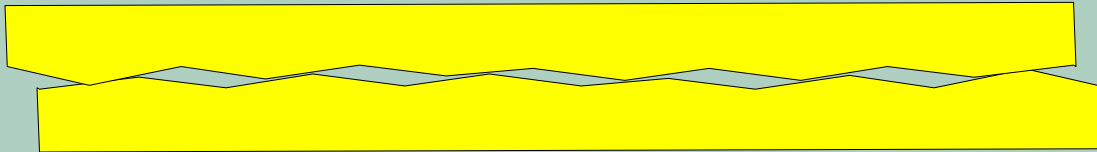


Extension to rough profiles



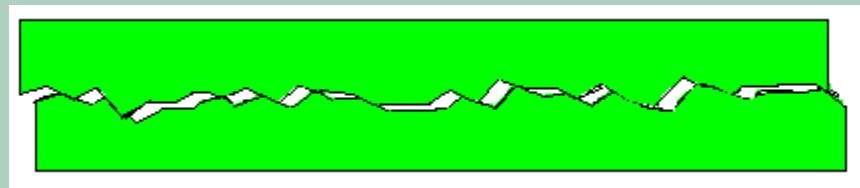
Same basic behaviour but more complex !

Regular triangular asperity profiles



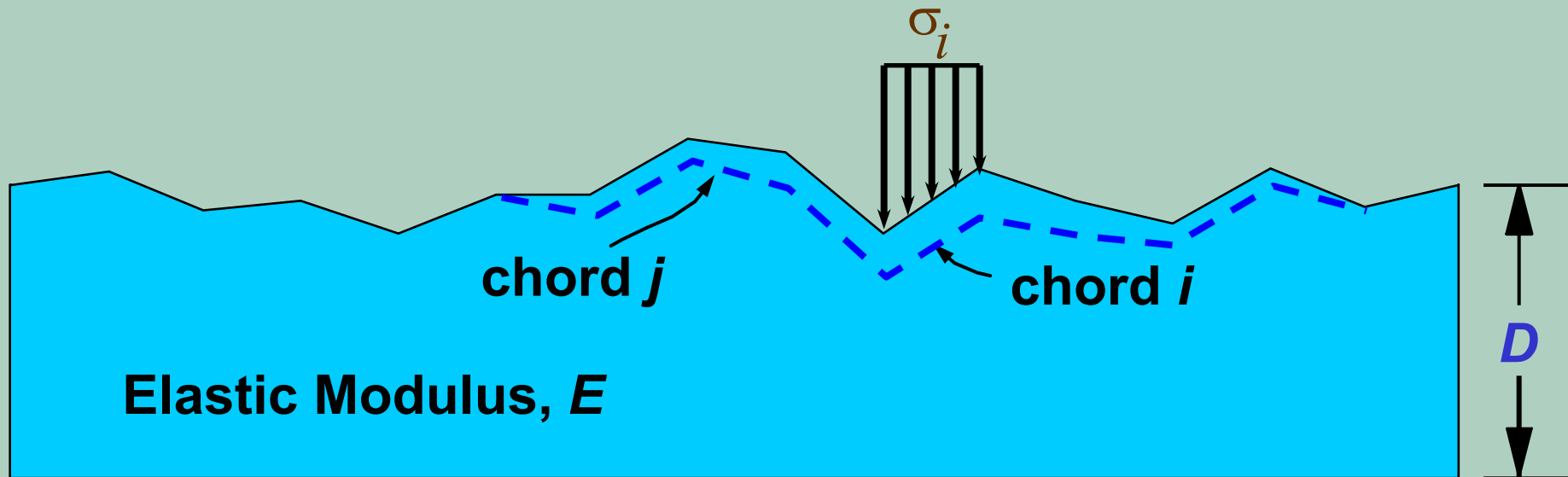
The conditions at every asperity are the same

Rough asperity profiles



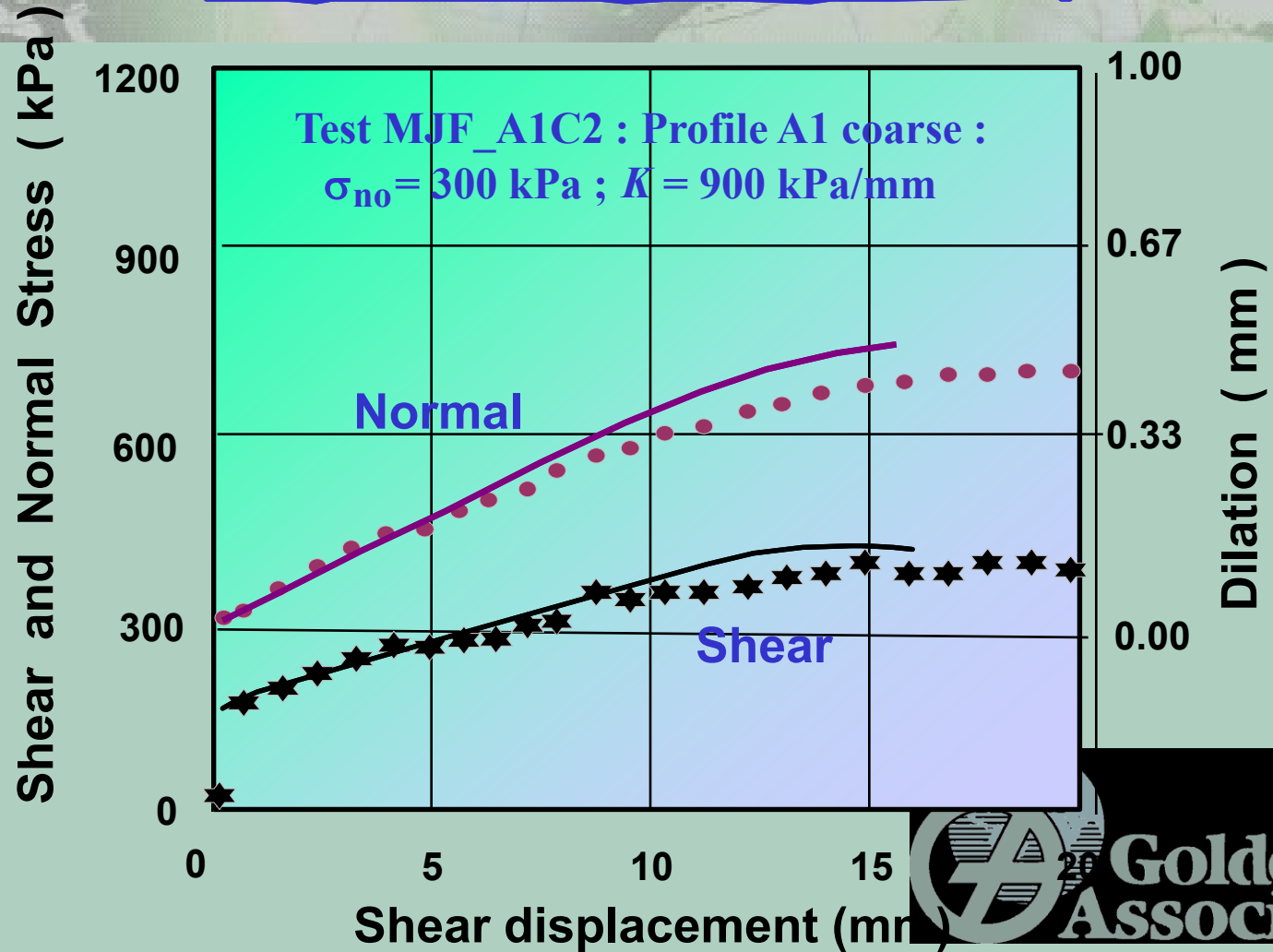
The conditions at every asperity are different

Asperity Deformation and Load Sharing

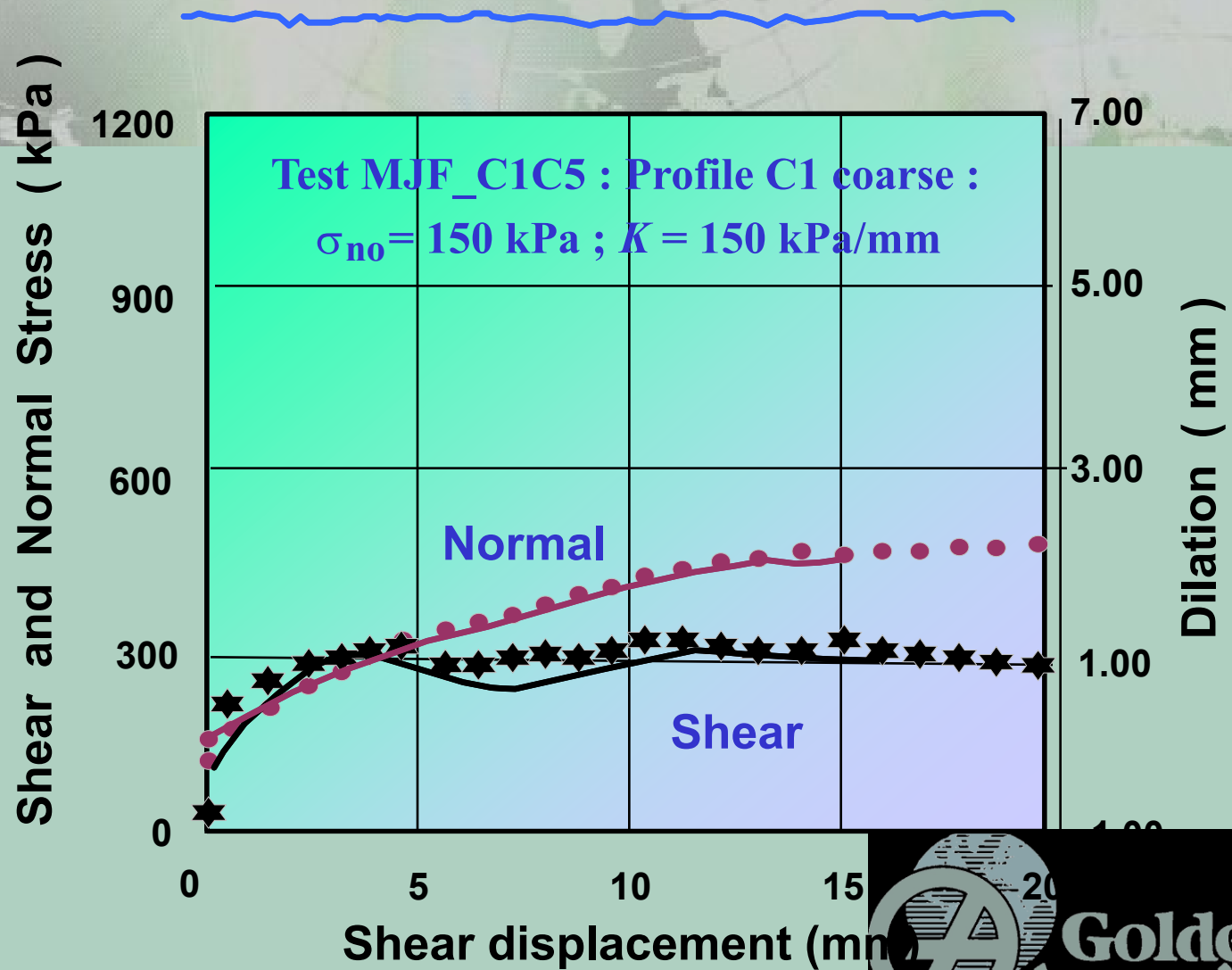


Laboratory Validation : fractal profiles

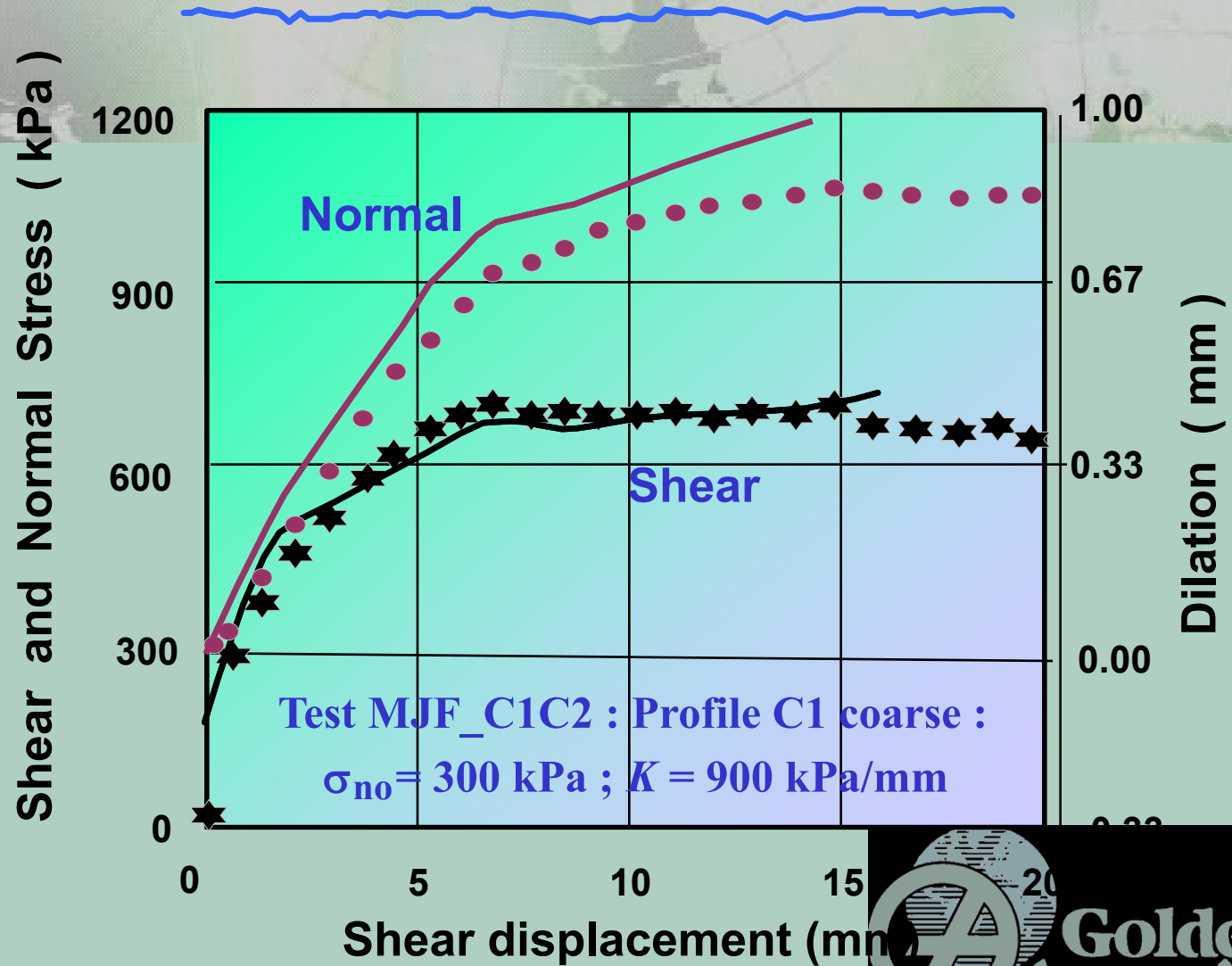
Class A Profile - measured vs predicted



Class C Profile - measured vs predicted

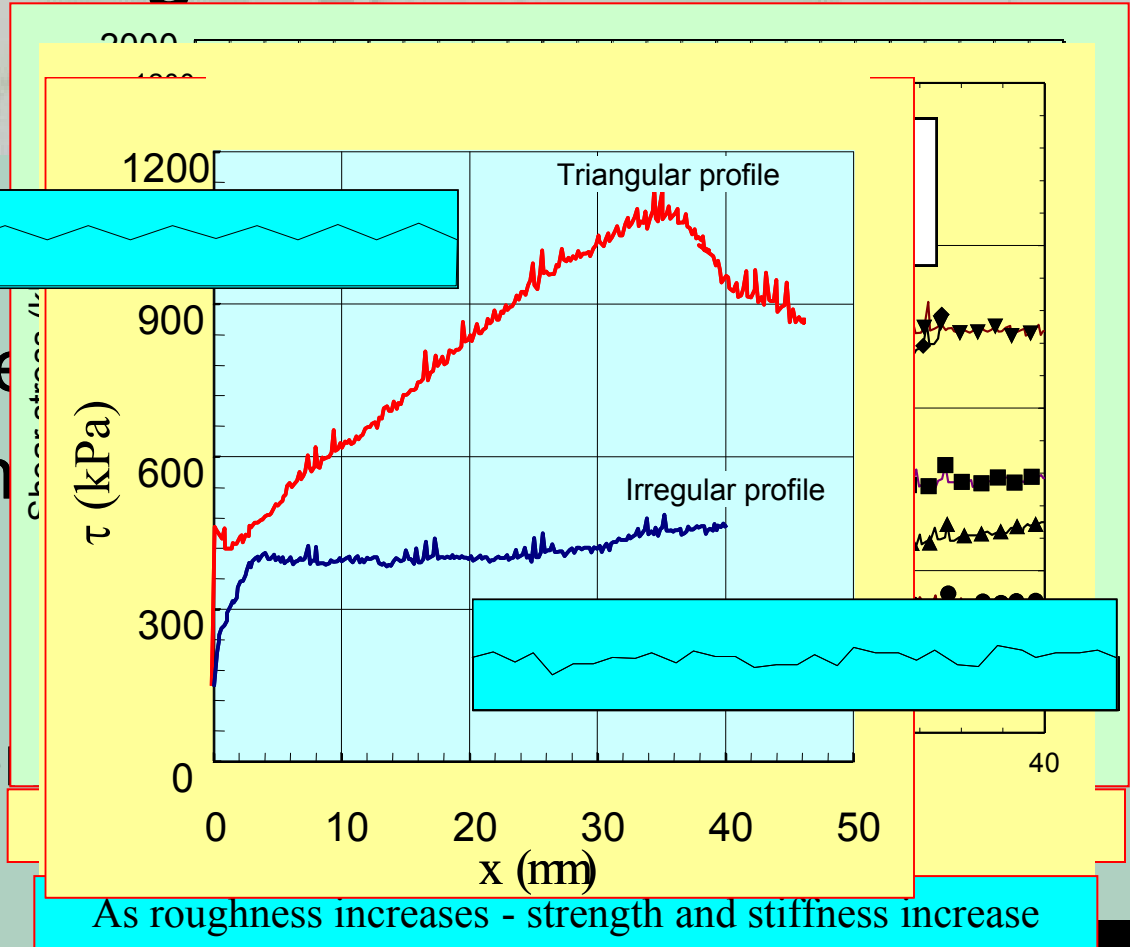


Class C Profile - measured vs predicted

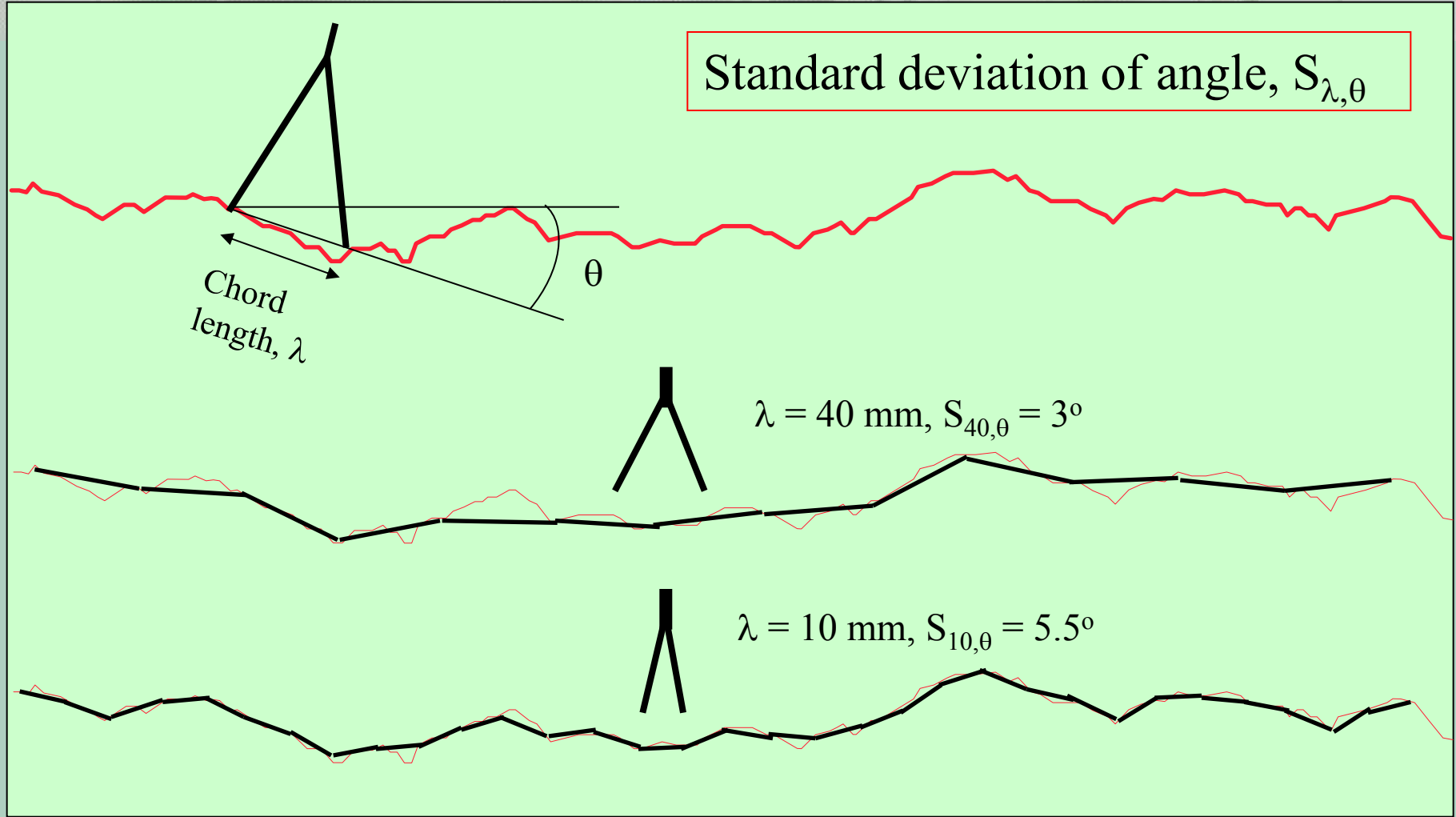


Understanding Roughness

- Impact is
- Shape dependent
- Optimal roughness for interlock but too much leads to dilation
- Scale dependent

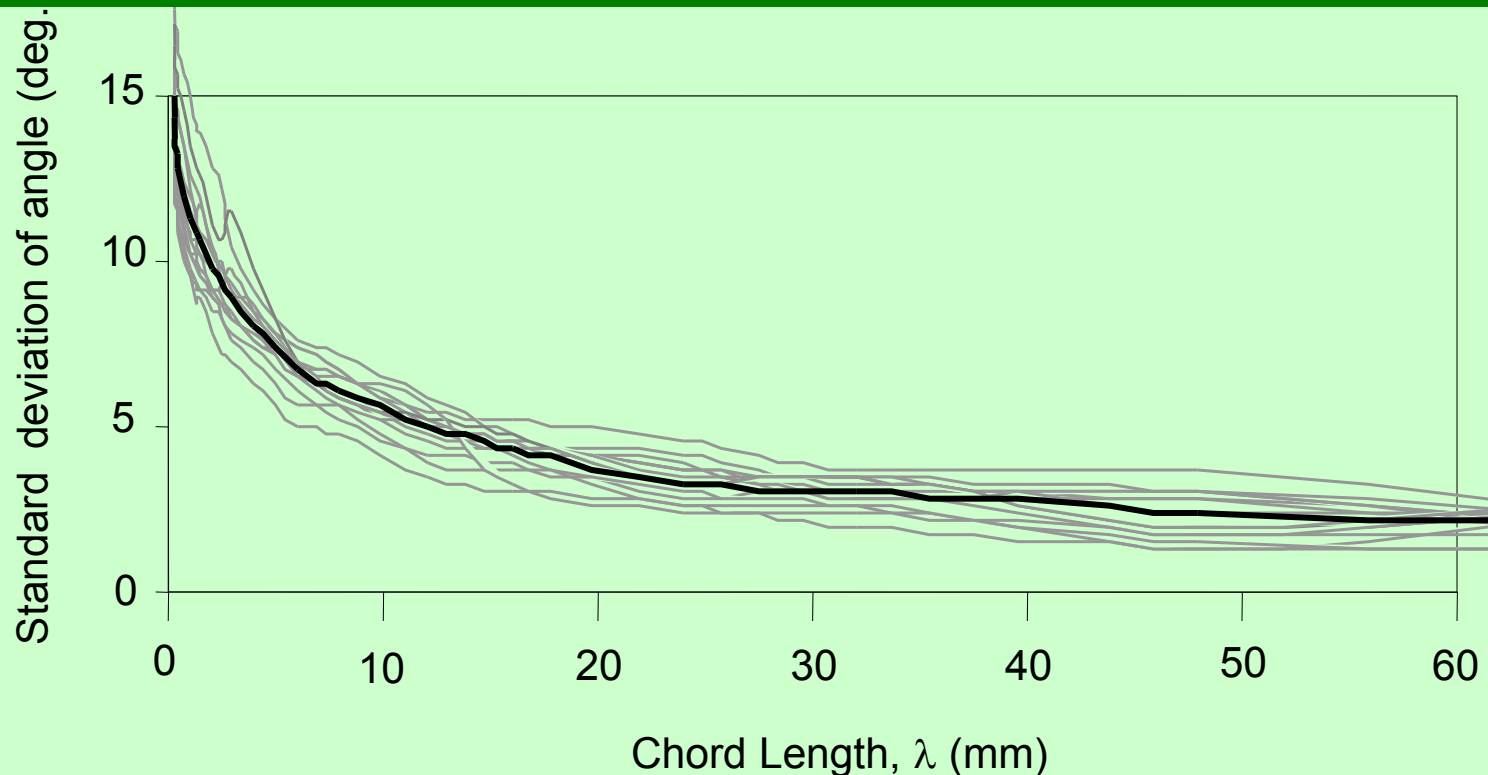


Scale effects



Roughness Parameter vs Scale

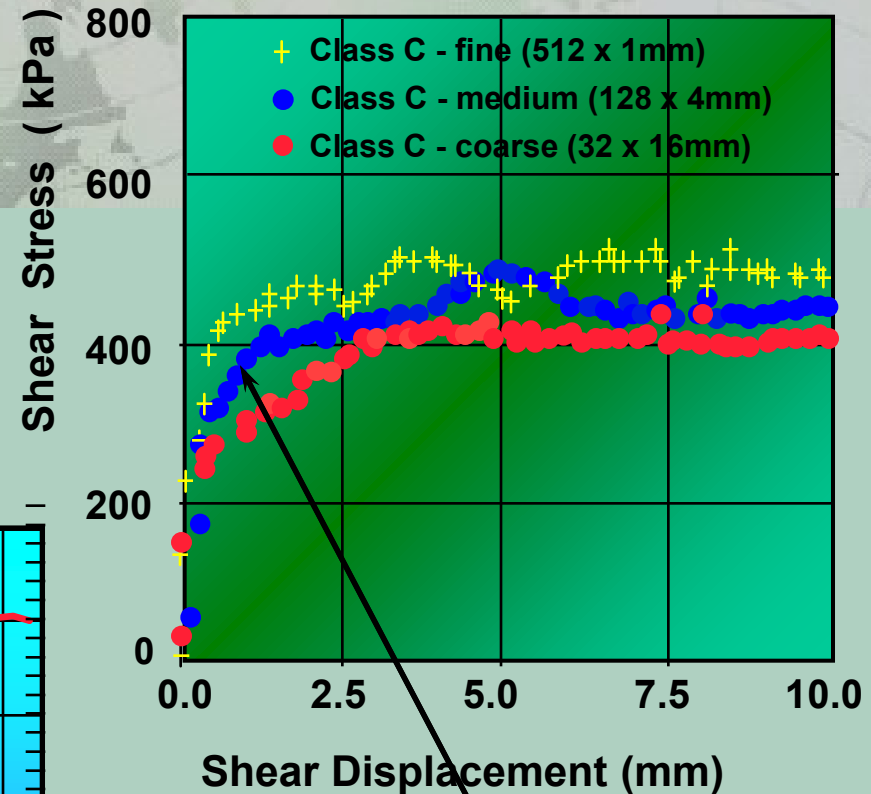
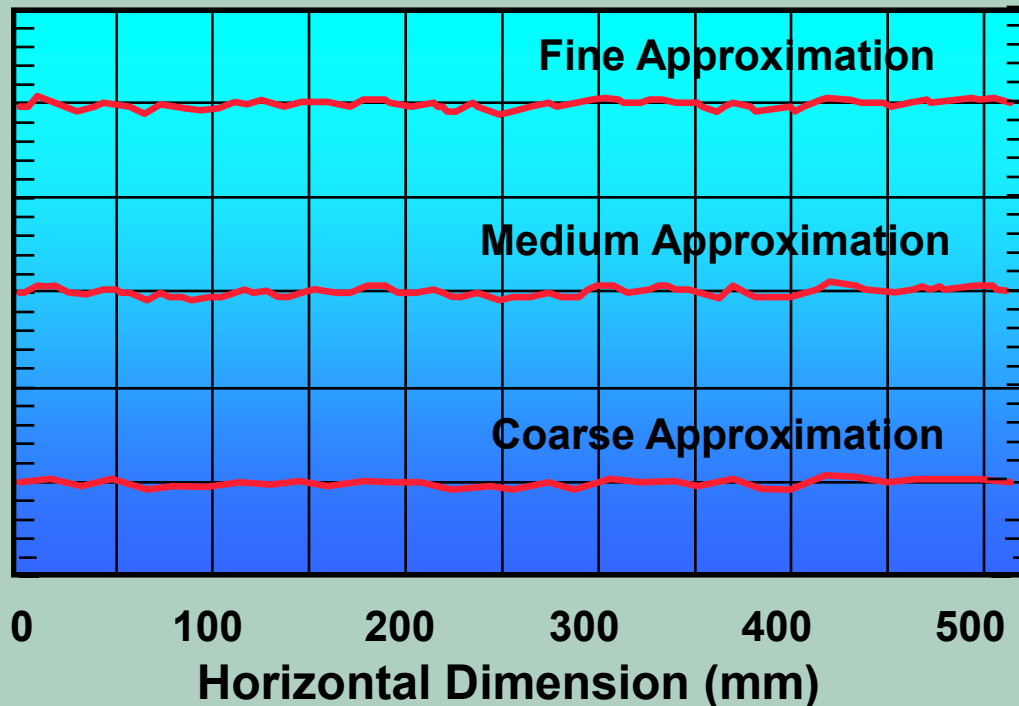
If we want to represent real roughness as a set of statistics - e.g. standard deviation of asperity angle (or height), what length scale (chord length) is appropriate?



Answer : All scales, but ...
in practical terms : it depends on the scale at which performance (often displacement) is being considered.

Some CNS direct shear test results

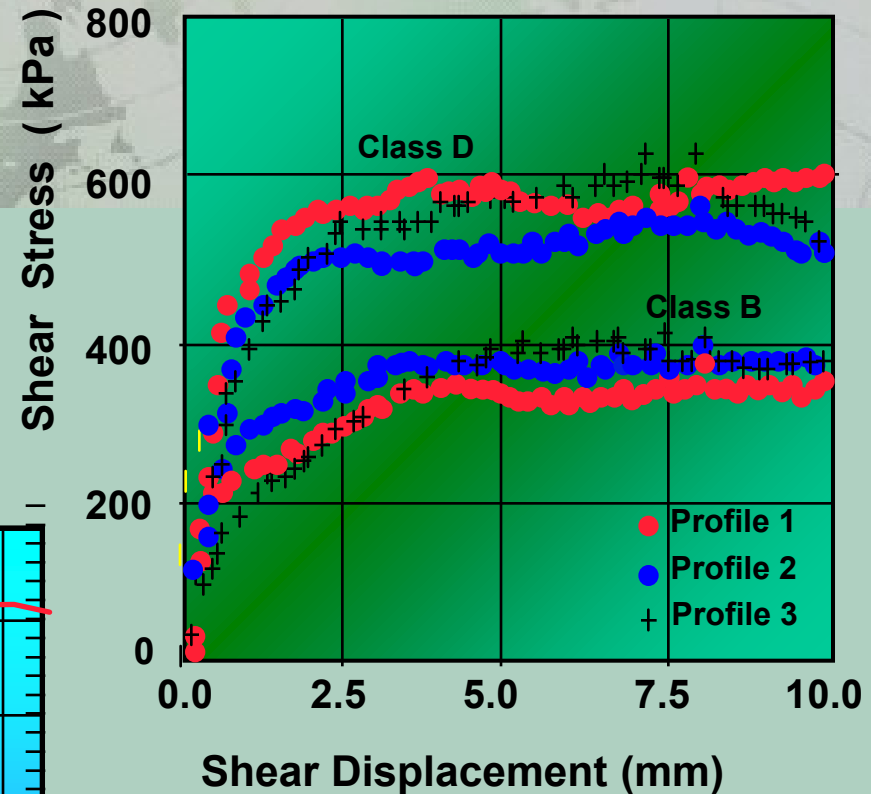
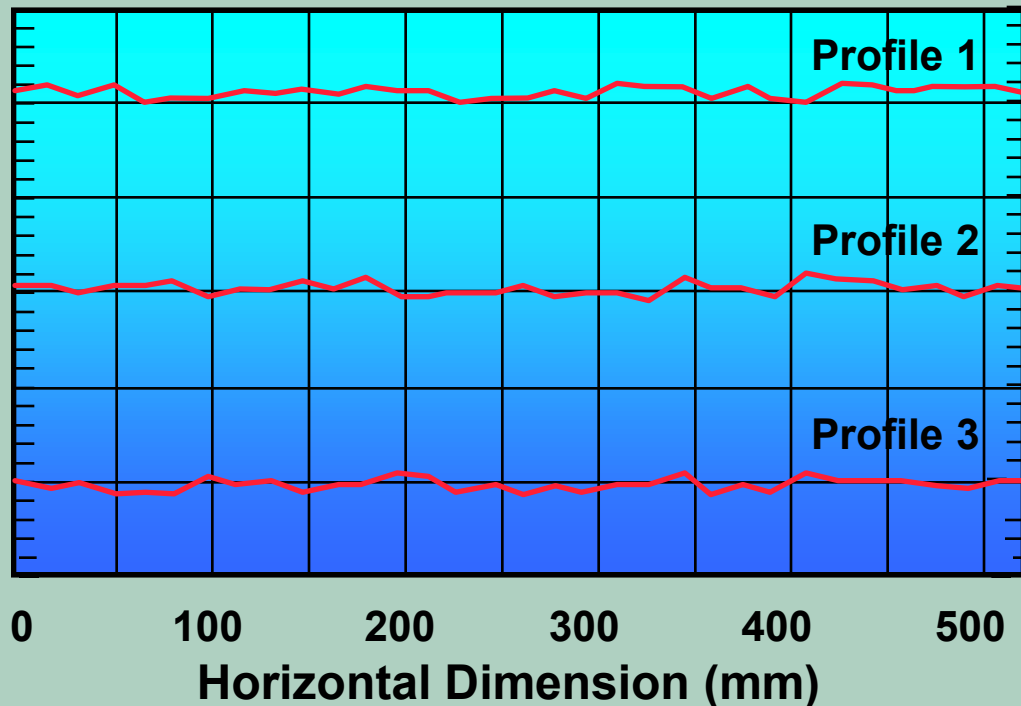
Coarse, medium and fine approximations of the same profile



- Stiffness is systematically higher for finer profiles
- Peak shear strength does vary systematically

Some more CNS direct shear test results

Profiles with different geometry but similar statistics also perform in an essentially similar manner.



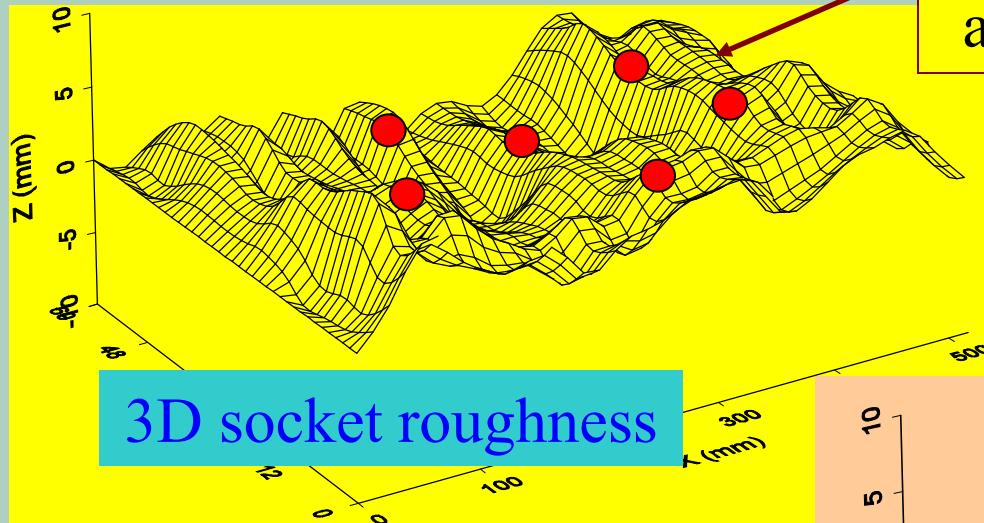
Impact of Roughness - Summary

- All scales of roughness important
- Small scale roughness impacts on initial stiffness
- Large scale roughness impacts on peak shear strength
- “Grooving” may not be advantageous



From laboratory test to rock socket

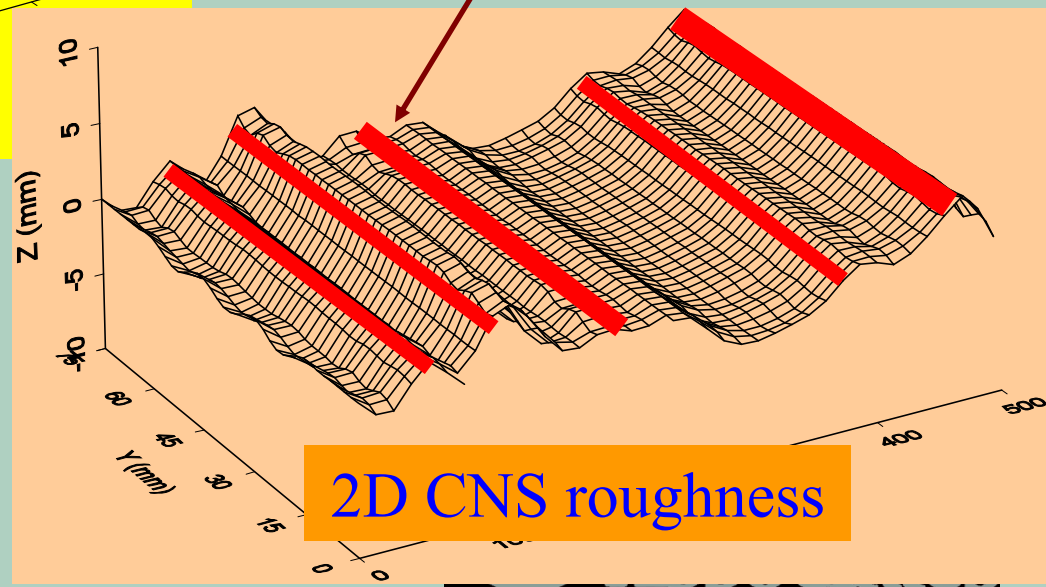
What are differences in roughness?



Patch contact
and loading

Strip contact
and loading

Rock socket roughness is
3D not the 2D approximation
used in CNS laboratory
testing



2D CNS roughness

From laboratory test to rock socket

Displacements and load sharing between asperities in laboratory sample and field

Take care when extrapolating CNS laboratory test results directly to field socket behaviour



Plane-strain : extensive but moderate deformation effect about loaded strip and along axis of sample. Thickness of sample is finite.

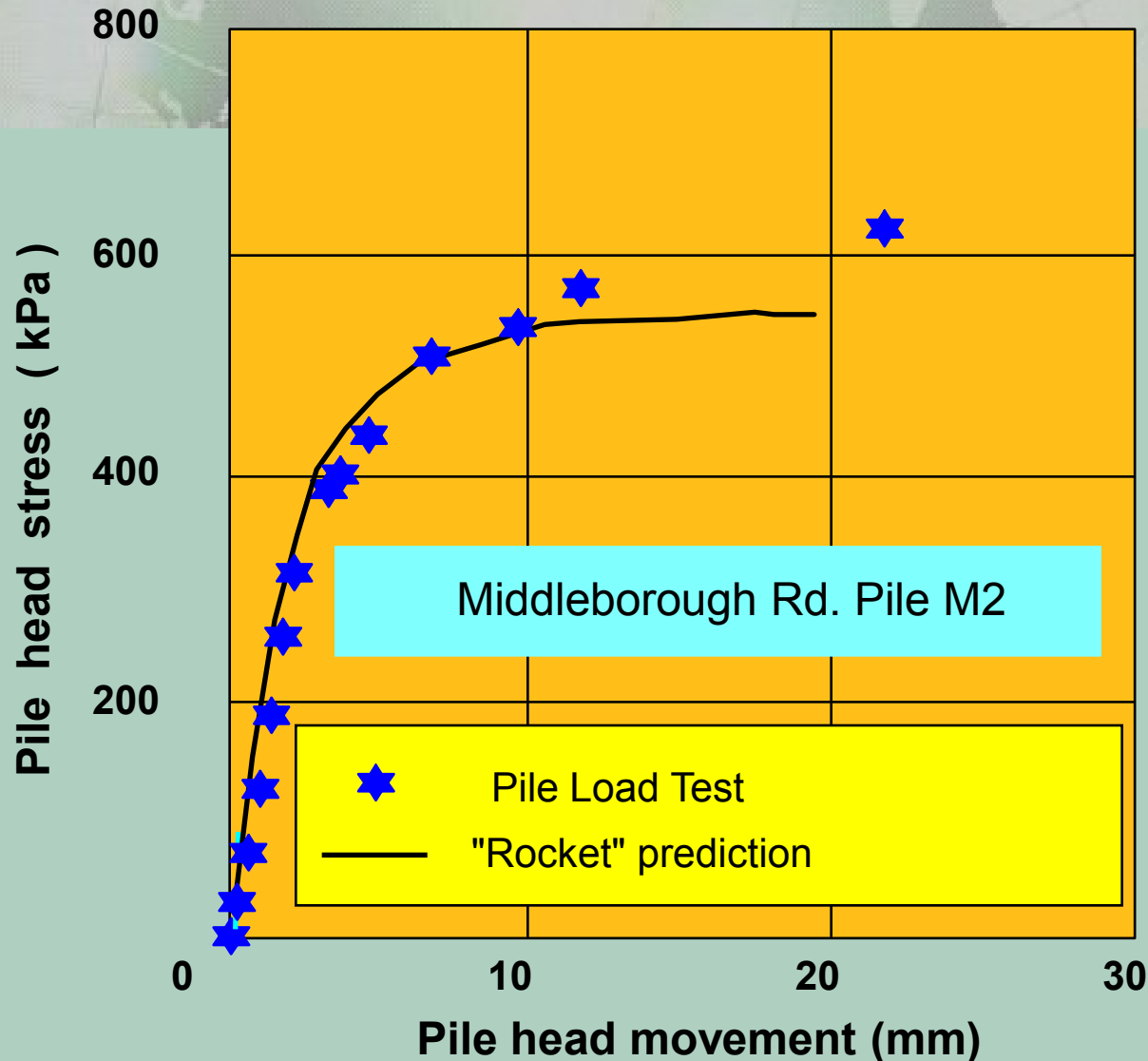
Patch-loading : intensive but localized effect about loaded patch both along pile axis and around circumference. Thickness is infinite.

ROCKET input parameters

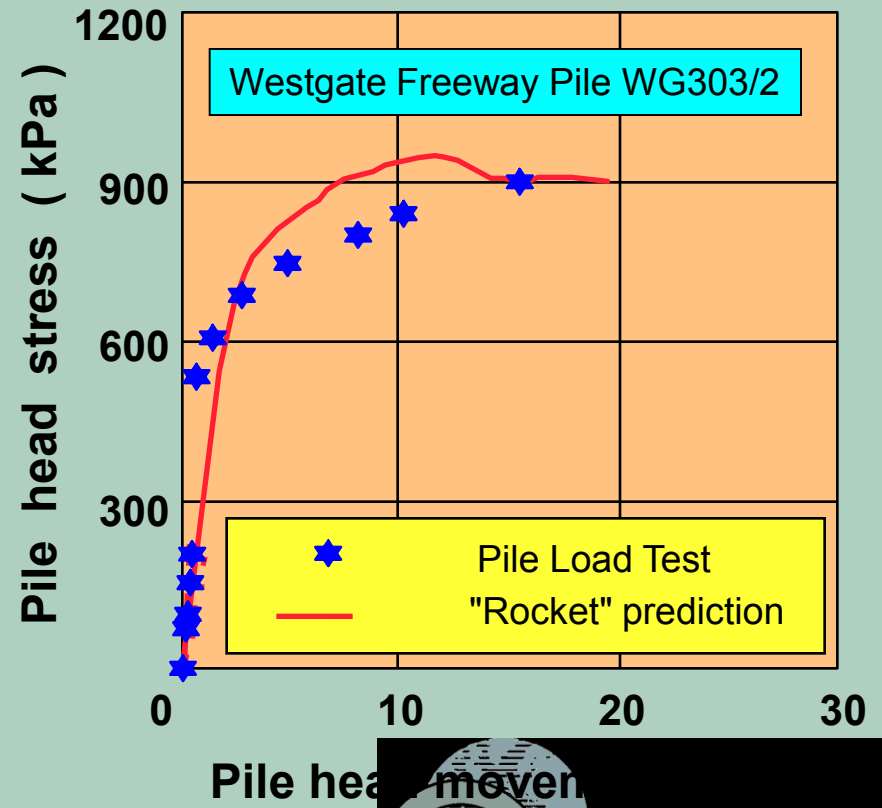
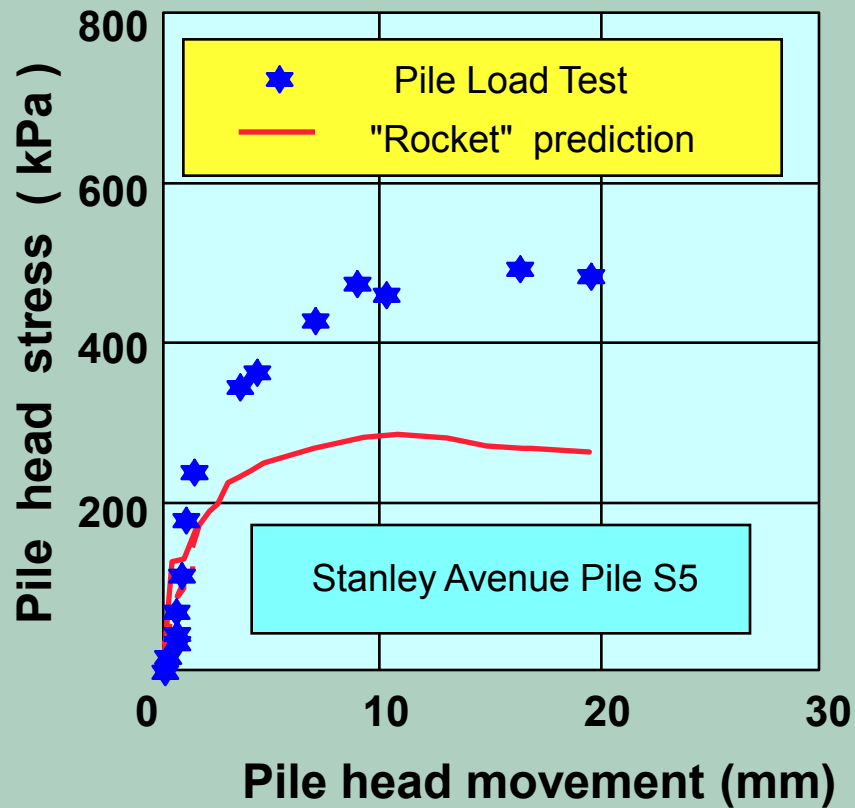
- Shear strength parameters - c' and ϕ'
 - drained triaxial tests
 - UCS and Hoek Brown
- Sliding friction angle - direct shear tests.
- Rock mass modulus and Poisson's ratio
 - pressuremeter tests.
 - triaxial tests, correction for jointing ?
 - moisture content correlations, correction for jointing ?
- Socket diameter (structural strength requirements)
- Socket roughness
 - direct measurements
 - back calculated from load tests
- Initial normal stress - estimated



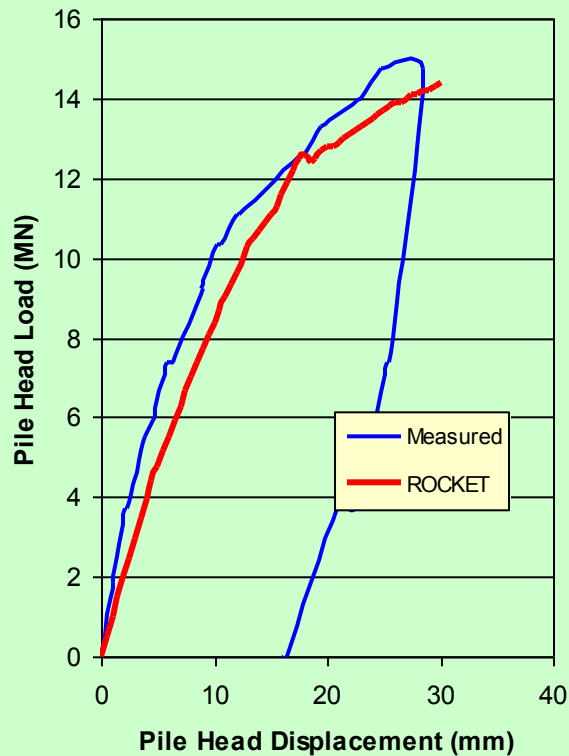
Field Validation



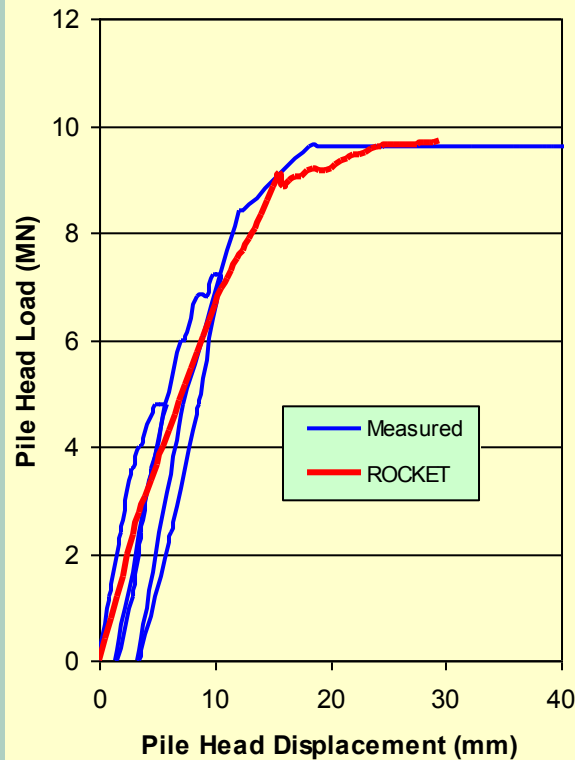
Field validation



Field validation



(a) TP1

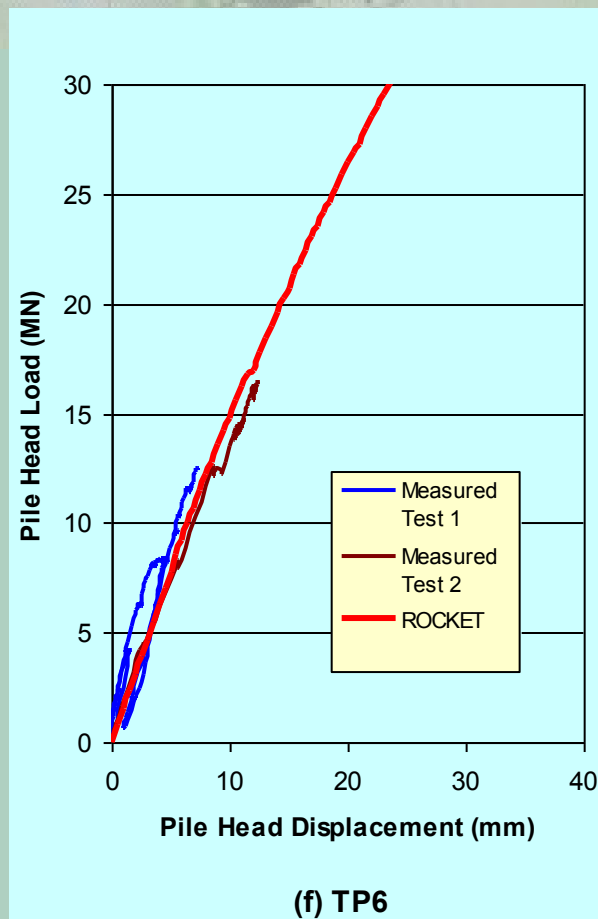
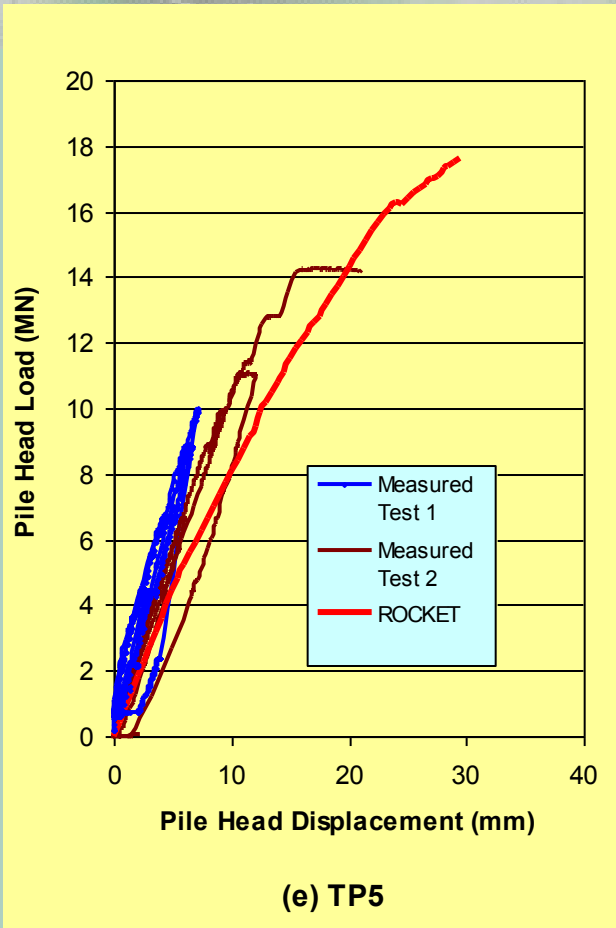


(b) TP2B

Bahrain

9 m sockets in
1 to 2 MPa
calcareous
siltstone

Field validation



Bahrain

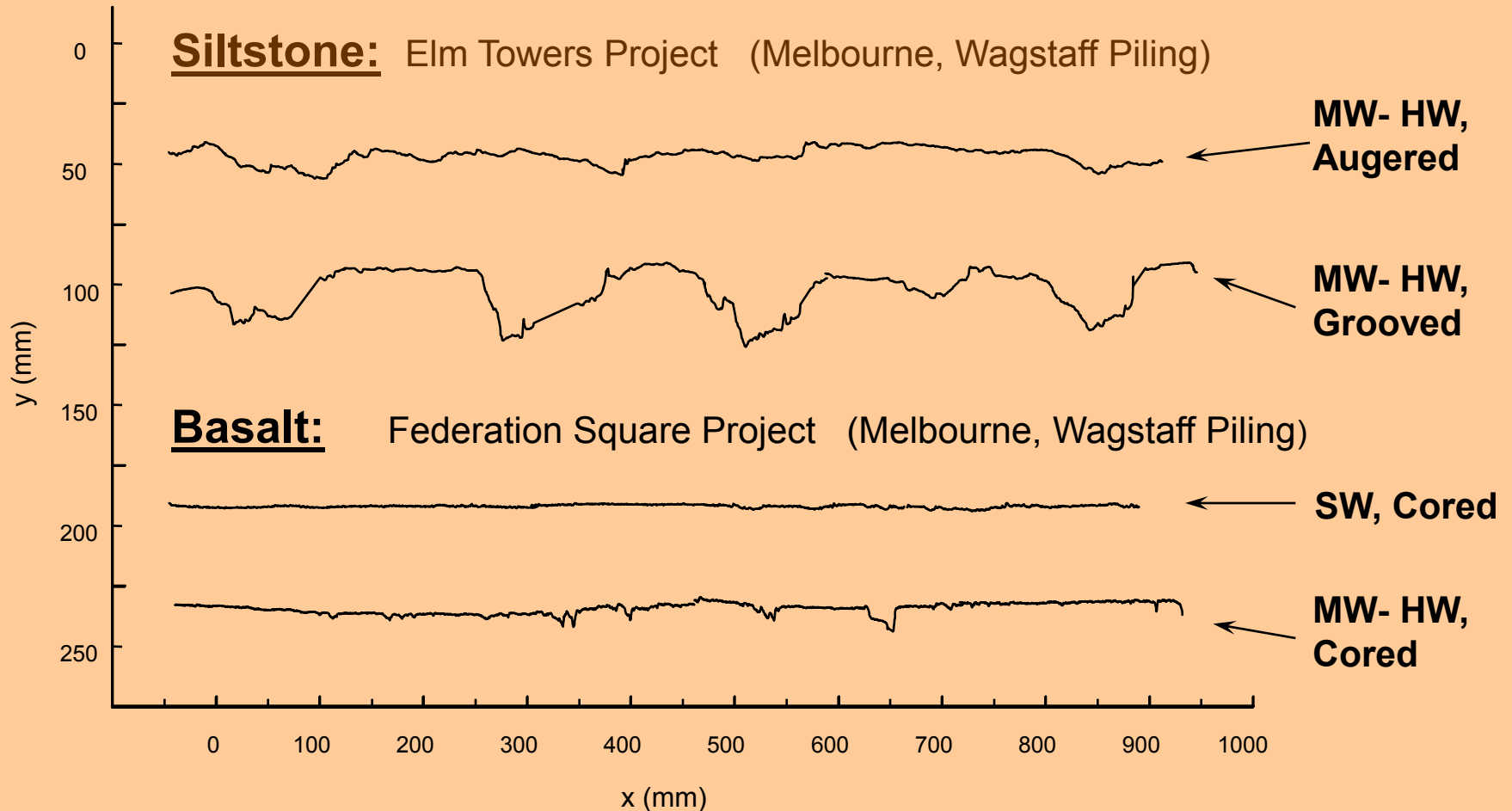
12 m and 15 m
sockets in 1 to 2
MPa calcareous
siltstone, toe of
15 m socket in
strong limestone

Field measurement of roughness

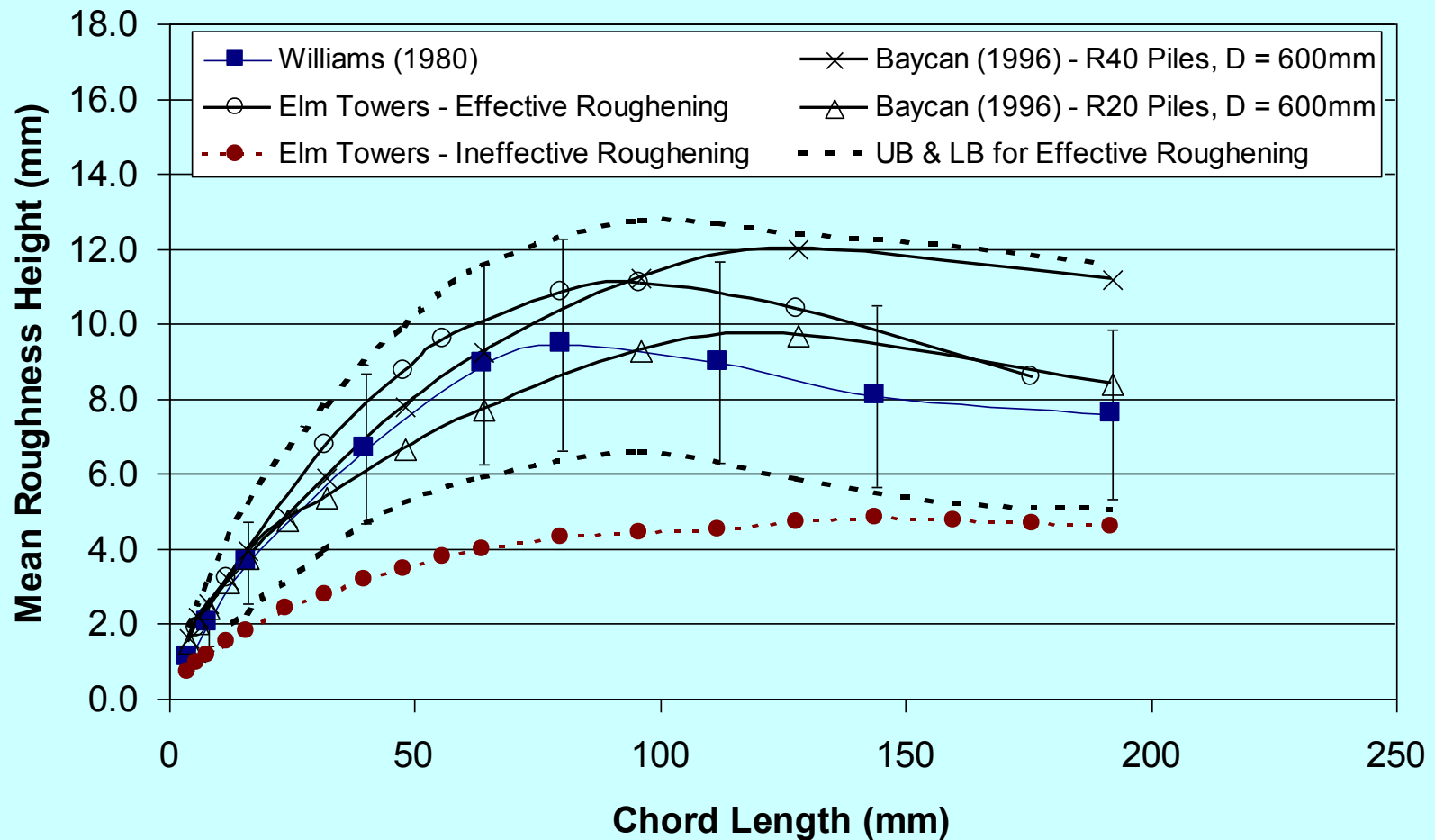


ates

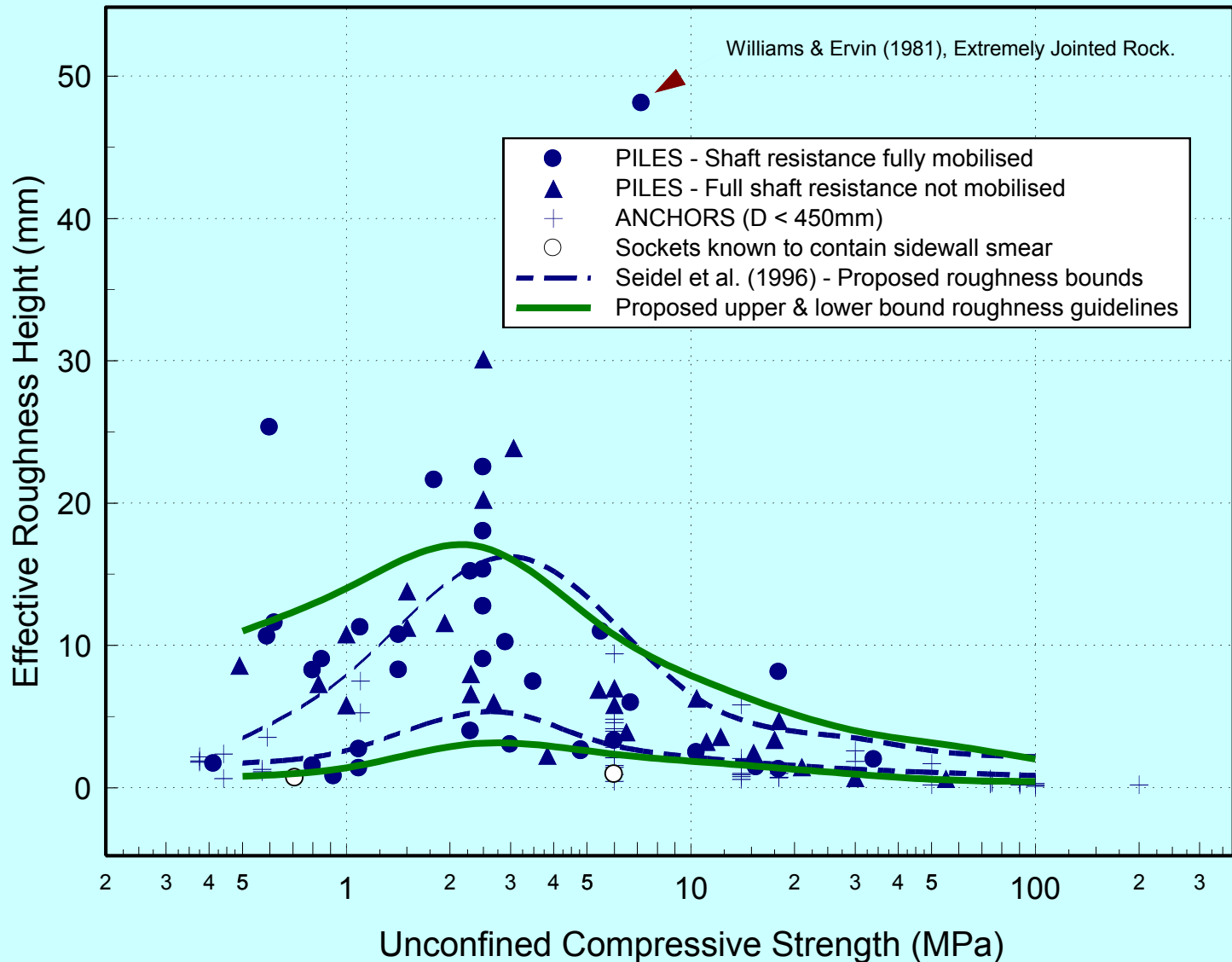
Example Profiles



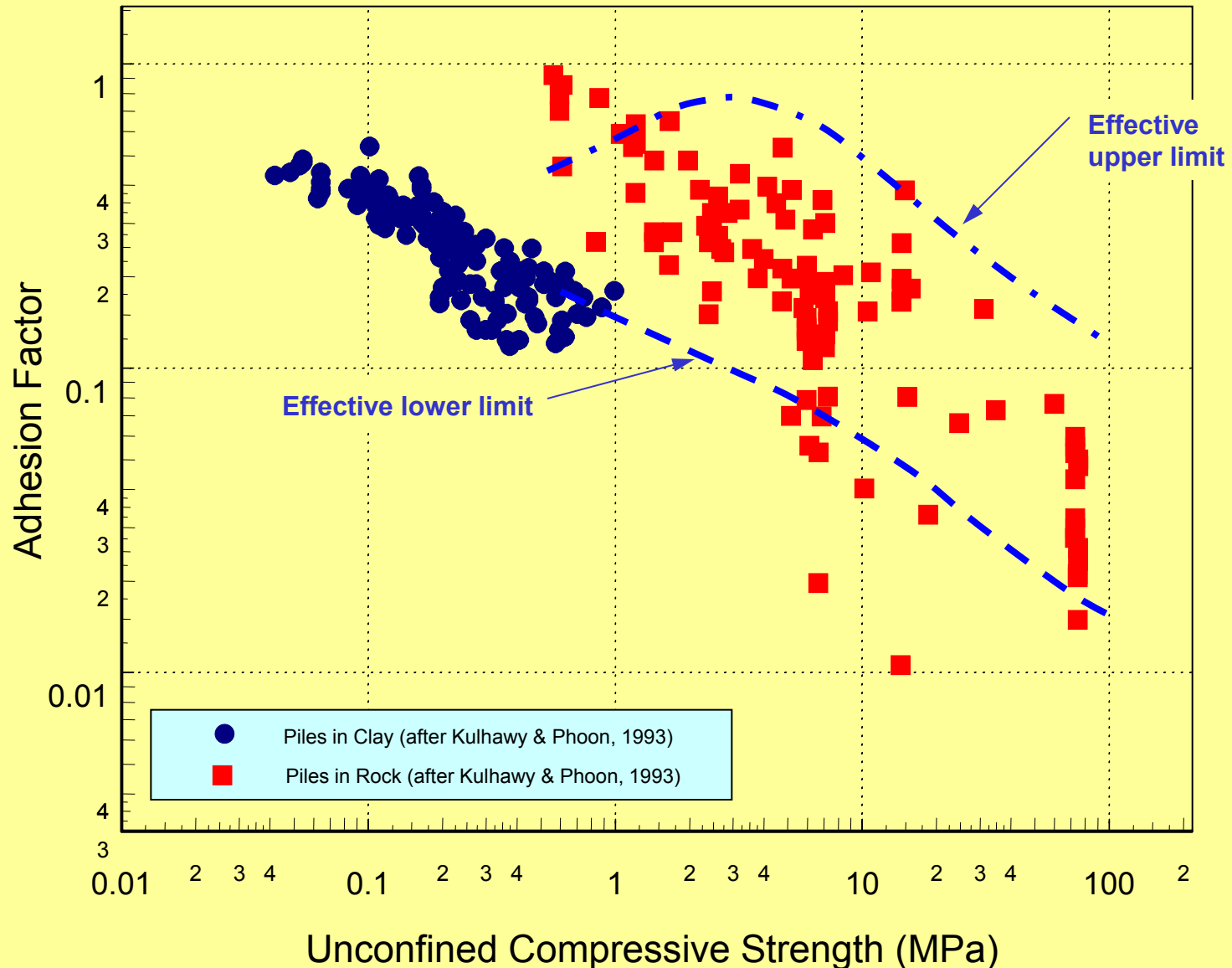
Some results in Siltstone



Back-calculated from pile load tests, $\lambda = 50$ mm



Does this explain the empirical load test data ?



Revisit : Parameters Affecting Shaft Resistance

Rock

- type, structure, and lithology
- strength
- stiffness

Affects normal stiffness and increase in normal stress with dilation

at strength and stiffness

Major impact on interface behaviour wrt stiffness and strength of response

Amount of

interaction between asperities

Construction

- socket diameter
- socket roughness
- socket cleanliness
- concrete placement
- contractor experience

Affects socket stiffness and normal stress, as well as load sharing between

May impact on soundness and integrity of pile

and interaction between asperities

Summary

- Understanding shaft resistance is of prime importance to predicting rock socketed pile performance
- Shaft resistance is highly dependent on rock properties, socket roughness and construction effects. Socket diameter also has an impact.
- Be aware of differences between CNS laboratory testing performance and field socket performance
- Sockets should be roughened – “grooving” may not be advantageous”

